SUBSURFACE INVESTIGATION OF LIQUEFACTION, IMPERIAL VALLEY EARTHQUAKE, CALIFORNIA, OCTOBER 15, 1979

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ABSTRACT

Subsurface investigations were made to study the effects of liquefaction caused by the October 15, 1979 earthquake (M=6.6), at Heber Dunes County Park and at River Park, Imperial Valley, California. Highly detailed profiles were constructed from cone penetration tests, standard penetration tests, large volume disturbed samples, and undisturbed tube samples. Liquefaction effects including sand boils, ground cracks, and lateral spreading occurred at the Heber Road site in a 5-m deep, loose, channel-sand deposit. The deposit occurs in an abandoned channel that is part of an ancient delta. The banks of the channel contain a moderately dense overbank deposit, and a dense point bar deposit. Other than a few sand boils over a buried pipeline, no liquefaction effects occurred in the denser sand deposits outside the channel. Hundreds of sand boils erupted on the flood plain of the New River at River Park. The sand boils at River Park originated from 3-m deep, loose, flood plain sands and silts, and from 6-m deep, medium dense channel sands. A 1.5-m thick, soft, clay layer is sandwiched between the two sand layers that liquefied. The behavior of the sands that did and did not liquefy are in general agreement with behavior predicted by standard engineering analyses.

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INTRODUCTION

An earthquake (M=6.6, Caltech Seismological Laboratory) struck the Imperial Valley on October 15, 1979. The earthquake was caused by right lateral movement on a 35-km long segment of the Imperial Fault (figure 1). Maximum right-lateral offset of 0.56 m occurred near Heber Dunes County Park (Real, 1979, p. 259). The earthquake generated liquefaction at 36 identified locations in the Imperial Valley, and caused damage to roads, canals and fields (Youd and Wieczorek, in press).

Two sites where liquefaction effects were particularly pronounced were selected for subsurface investigation (figure 2): a lateral spread that crossed Heber Road 16 km southeast of El Centro, California; and River Park, in the southwest part of Brawley, California, where hundreds of sand boils erupted during the earthquake. Between December 3 and 19, 1979, and between January 5 and 30, 1981, subsurface investigations were made at Heber Road and at River Park. These investigations included, cone penetration tests (CPT), standard penetration tests (SPT), continuous, disturbed sampling, and thinwalled tube samples.

METHODS OF INVESTIGATION

Cone Penetration Test (CPT)

A linear array of cone penetration soundings was made to determine (1) a subsurface cross section of sediment layers, (2) the types of sediment penetrated and (3) an estimate of the compactness of granular layers

penetrated. A mechanical friction-cone penetrometer was used to determine point resistance and sleeve friction values. The values are obtained by following the procedures outlined in ASTM standard D-3441-75T (ASTM, 1978, p. 450). The penetrometer used in this study was a Begemann friction-cone (figure 3), the cone has a 60° point with a base area of 10 cm². The friction sleeve has a surface area of 150 cm². A Mobile B-50 drill rig was used to advance the penetrometer using a conversion kit described by Drnevich (1974, p. 125). Measurements were made at 20-cm intervals by alternately advancing the cone and the friction sleeve (figure 3). The cone measures point resistance (q_c) , the friction sleeve measures side friction (f_s) . Point resistance data is used to estimate relative density (Schmertmann, 1978a, p. 40). The friction ratio $(f_s/q_c=R_f)$ is used to identify soil types (Schmertmann, 1978b, p. 6). In general, cohesionless sand and silt have friction ratios less than 3 percent. Cohesive clayey silt and clay have friction ratios greater than 4 percent (Schmertmann, 1978b, p. 7; Sanglerat, 1972, p. 211).

Standard Penetration Test (SPT)

Standard penetration tests were made at selected points in the soil profile to determine N-values (blows per foot) for evaluating liquefaction susceptibility and to obtain samples for soil classification. The equipment and procedures used in this investigation generally follow the recommendations of ASTM standard D1586-67T (ASTM, 1978, p. 235). The procedure was modified to use hollow-stem auger. A hollow-stem auger (25 cm o.d., 10 cm i.d.) was used to drill holes and act as casing. The step by step procedure for conducting the tests is shown in figure 4a. A pilot bit was used inside the auger to drill to the first sampling depth. As the auger advanced to within

about 15 cm of the sampling depth the rate of advancement was reduced to lessen the disturbance to soils below the auger. After reaching the proper testing depth, the auger was lifted 10 to 15 cm and suspended from a fork at the surface. Water was added to create a hydrostatic head and prevent sand from being sucked into the auger as the pilot bit was removed. In instances where a pilot bit was not used, the depth of the hole was measured to determine the amount of sand inside the auger. If sand was inside the auger a hose was lowered to the bottom of the hole to flush out the sand. On the end of the hose was an attachment for directing flow outward and upward rather than downward. After flushing sand out of the auger, the sampler was lowered to the bottom of the hole. The drill rod was then marked with 6. 12, and 18 in (15, 30, and 45 cm) reference lines and the sampler driven 18 in (45 cm) with a 140 lb (63.5 kg) Mobile safety hammer falling 30 in (75 cm). The hammer was lifted and dropped using a reversing winch that allows the hammer to free fall. The sampler was driven 6 in (15 cm) for seating. The number of hammer blows required to drive the sampler the next 12 in (30 cm) was the penetration resistance, N. After the test, the auger was advanced to the next testing depth with the sampler acting as a pilot bit. Near the testing depth drilling was slowed to minimize soil disturbance. When the testing depth was reached the auger was lifted 10 to 15 cm and suspended with a fork. The split-spoon sampler was removed from the hole as noted in step 4. A clean sampler was then lowered to the bottom of the hole, any change of hole depth noted, and the driving and drilling procedures repeated (steps 4 through 6, figure 4a).

Although we have not made extensive comparisons of standard penetration results obtained by the hollow-stem auger technique described above with other SPT techniques, we do have data from parallel tests at one site near Salinas,

California. At that site, we conducted SPT tests in two parallel borings and also made cone penetration soundings. Martin and Douglas (1981, Figs. A34-A41) conducted SPT tests in 8 borings at the same site. In 4 of the borings they used a conventional rope-around-a-cathead technique, and in the other 4 borings they used an automatic trip hammer. All of the Martin and Douglas tests were conducted in rotary drilled holes filled with drilling mud. Averaged blow counts from the three sets of SPT tests plus USGS cone resistance data are plotted on figure 4b. The N-values obtained by the hollow-stem-auger technique are generally consistent with, but average about 25 percent less than the values obtained with the automatic trip hammer. SPT values obtained with the conventional procedure are all considerably greater than those obtained by the hollow-stem-auger or automatic-trip-hammer techniques. The average ratio of the conventional to the hollow-stem-auger SPT resistance is 2.5. The cone penetration resistance and blow counts from the automatic trip hammer and hollow-stem-auger techniques all increase essentially linearly with depth, indicating a rather uniform relative density of the deposit to a depth of about 10 meters. N-values from the conventional test generally increase with depth, but have some variations not seen in the other records. Also, the latter N-values have a wider range of scatter than the values obtained by the other methods. These data indicate that the conventional SPT test does not produce as consistent results as the more automated procedures, even under the carefully controlled conditions maintained during the Martin and Douglas investigation.

Continuous, Disturbed, Sampling

Continuous, disturbed samples were taken for visual classification and laboratory grain-size analysis. Disturbed samples 1.5 m long were obtained by

"cork-screwing" solid-stem augers into the ground. In this procedure, a solid-stem auger 6 in (15 cm) in diameter was advanced into the ground at a rotation rate that minimized soil movement up the auger (figure 5). After an advance of 1.5 or 3.0 m, the augers were pulled vertically out of the ground without rotation. Field classification and photographs were made while the sediment was on the auger. Changes in texture and color could be seen very well by this technique. Unfortunately, most of the structure and fine layering was disturbed.

When drilling in soft clay or loose sand only a few centimeters of sample displacement occur along the augers. In dense sands or very stiff clays, upward displacement was as much as 0.50 m. When used in conjuction with CPT and SPT, sediment can be easily located in its proper stratigraphic position. In addition to the disturbed samples, six Shelby tube samples were taken at the Heber Road site for identification of small-scale sedimentary structures.

Soil Classification

Field and laboratory tests were used to classify soils according to the Unified Soil Classification system (D2487-69, ASTM, 1978, p. 325). Field classification included general descriptions of texture, stratification, and color (using the Munsell soil color chart). Laboratory tests included grain size distribution and Atterberg limits. Grain-size measurements were made using procedures prescribed in ASTM standard D422-63 (ASTM, 1978, p. 71). Atterberg limits were determined using procedures in ASTM D424-59 (ASTM, 1978, p. 86) and D423-66 (ASTM, 1978, p. 82).

The Imperial Valley occupies the northern part of the Salton Basin which is a northward extension of the crustal rifting that opened the Gulf of California. The basin is tectonically active as evidenced by movement on the Imperial and Brawley Faults. The basin ranges from 5 to 130 km in width and is about 200 km long. The central part of the basin contains the Salton Sea, 70 m below sea level. The basin contains up to 6,100 m of Tertiary and Quaternary marine, continental and lacustrine sediment. Pleistocene and Holocene sediment (Holocene sediment is as thick as 100 m in some areas) has been derived from two sources; (1) the Colorado Plateau, through the Colorado River and its distributaries which include the Alamo River, and (2) local hills and mountains bounding the basin. Sediment from bounding hills and mountains has carried into the basin through the New River system. The Holocene depositional history includes thick lacustrine deposits, channel-fill deposits, alluvial fans and eolian deposits.

Channel-fill deposits of the Alamo River form shoestring sand bodies within the lacustrine mud and deltaic sand facies. The channel deposits range from 3 to 20 m thick, and 65 to 300 m wide (Van De Kamp, 1973, p. 835). The Alamo and New Rivers contain distinctly different sediment because the rivers have different source areas. The Alamo, supplied by the Colorado River, contains red-brown, well-sorted fine sand and clay, whereas local sources supply the New River with yellow-white, medium to coarse, moderately sorted sand and pale green and tan clay. The juxtaposition of the channel, delta, and lacustrine deposits produces rapid changes in vertical and lateral profiles.

Between 300 and 1600 years ago natural flooding from the Colorado River filled the Imperial Valley and created Lake Cahuilla (Van De Kamp, 1973, p. 829). Fine-grained muds deposited in Lake Cahuilla commonly interfinger with sand beds. Variations in lake level caused sand to prograde over the muds. The source of the muds can be identified from color. The brown-red muds originated from the Colorado River system, whereas pale green and tan muds originated from local valley margins (Van De Kamp, 1973, p. 832).

Before agricultural modification, small lakes filled many depressions along river channels that meandered across the old lake bed. Sediment accumulated in these small lakes as deltas similar to the modern deltas in the Salton Sea (Van De Kamp, 1973, p. 834). The position of the deltas are controlled by; location of depressions, course of channels, and fluctuations in lake level. Van De Kamp (1973, p. 834) reports that there probably are numerous small deltaic sequences in the southern part of the Imperial Valley.

One of these deltas, fed by a now abandoned channel, occurs west of the Alamo River between Holtville and the Mexican border. Archaelogic and stratigraphic evidence indicate that Lake Cahuilla covered this area 300 to 400 years ago (Sharp, in press). The delta is identified by topographic contour lines that show a lobe of higher ground (Youd and Wieczorek, in press) and by soil surveys (Strahorn and others, 1924) that indicate the lobe of high ground is composed of sandy soils (figure 6). Aerial photography, from 1937, of the delta shows one major remnant stream channel, outlined by levees and vegetative photolineaments, running down the axis of the delta (figure 7). Since 1937, most of the channel features have been obliterated by land leveling and cultivation.

Liquefaction effects produced by the 1979 earthquake were concentrated within the delta and particularly in the naturally and artifically filled channel. The Heber Road site is located across the relict channel.

Heber Road

At Heber Road a lateral spread formed in the naturally and artifically filled channel described above. The spread disrupted the pavement on Heber Road 1 km east of Mets Road and shifted the roadway and an adjacent canal 1.2 m southward. The lateral spread is about 160 m wide and 100 m long. Arcuate ground cracks and scarps formed around the margins and across the interior of the failure (figure 8). Scarps up to 0.9 m high formed at the southern edge of the road. Many sand boils erupted on the lateral spread, primairly along cracks north of the road. The eastern edge of the channel is marked by ground cracks. East of the former channel no ground cracks or ground displacements occur although sand boils erupted over a buried drain. No cracks or sand boils formed west of the channel.

In December 1979, and January 1981, subsurface investigations were made across the lateral spread and parallel to Heber Road to define the sediment profile and to classify the soil properties. Figure 9 is a photograph of the site south of Heber Road. Figure 10 shows the location of the holes in relation to the failure. The preliminary subsurface investigation in December 1979, consisted of 8 static cone penetration (CPT) soundings 30 m apart (holes 1 through 8) and 2 CPT soundings (holes 9 and 10) 4.5 m apart near hole 7 to define the east margin of the old channel. Continuous, disturbed samples were taken from holes 1, 4, and 7. Standard penetration tests were made at holes 1, 4, 6, and 7. Shelby tube samples were taken from holes 1 and 7. The Heber Road site was visted again in January 1981, to make shear wave velocity measurements. This work was performed in conjunction with Kenneth Stokoe

(University of Texas, Austin) under USGS Research Contact no.

14-08-0001-19777. In this study 3 lines of tests were set up parallel to the relict channel; 1) one site was located west of the lateral spread, 2) one site was located within the lateral spread, 3) one site was located east of the lateral spread. One CPT was made at the ends of each line. One SPT was made near the middle of each line. Approximately 40 thin-walled tube samples were taken from the 3 sites for further testing by the University of Texas and Woodward-Clyde Consultants, Clifton, N. J., another participant in the USGS contract noted above. Figure 11 is an aerial photograph that shows all drilling locations at Heber Road.

River Park

Hundreds of sand boils and numerous small (less than 5 cm wide) ground cracks developed in a large graded area along the east side of the New River at the southwest edge of Brawley. The graded area called River Park contains a rodeo ground, picnic area, stock pasture, and an unpaved parking area. The eruption of sand boils flooded the parking area and covered parts of that area, the stock pasture, and the picnic ground with water and sand. Water continued to seep from some sand boils for 2 weeks following the earthquake.

A rotational slump developed in a zone of artifical fill in part of the picnic area near the New River. The slump scarp runs 30 m along the river and extends 5 m to 15 m inland from the rivers edge. The height of the scarp varies from 1.2 m at its northern end to a negligible height at its southern end. where it merges with small cracks that were very common along the river. At the northern end of the slump a dome-like toe buldged out at river level.

At River Park 5 CPT soundings were made across the flood plain at intervals of 60 m (figure 12). Six additional soundings at intervals of 10 m

were made across the slump near the New River. Standard penetration tests were made at hole 1 on the flood plain and at hole 6 on the slump. Cork-screw samples were obtained at two of the CPT sounding sites on the flood plain and at four of the sounding sites near the slump. In January 1981, the River Park site was revisted. Four CPT were made, 3 were on the slump, north of the previous tests, and one was near hole 4 on the flood plain. Standard penetration tests were made at hole 13, near the slump, and near hole 15, on the flood plain. The location of all the tests is shown in figures 13 and 14.

RESULTS

Heber Road

As noted above, the investigation of the lateral spread at Heber Road included 16 CPT soundings, seven SPT soundings, four holes for continuous, disturbed sampling, and five holes for undisturbed shelby tube samples (figure 10). Tables 1 through 17 contain CPT and SPT data, geologic logs, soil classifications, grain size analyses, and Atterberg limits. The samples, and penetration data were used to construct the profile in figures 15 and 16. As shown in figures 15 and 16 the upper 5 m consists of 3 different units of sand and silty sand, units A_1 , A_2 , and A_3 . The average penetration resistance and the relative density of the units are shown in figure 17.

Fluvial Units

Unit A_1 . Unit A_1 occurs on the west side of the channel and was penetrated by soundings and borings at holes 1, 11, and 12 (tables 2, 12, and

13). The sediment in the unit is dark brown, dense to very dense, well sorted, very fine-grained sand. The unit shows a well defined upward decrease in grain size. The unit is wedge shaped, tapering from 5 m thick at hole 1 to less than 0.5 m thick in hole 4. The sand consists of about 95% rounded to subrounded quartz and feldspar grains. The other 5% of the sediment consists of heavy minerals, mica, and fragments of organic material. No gastropod shells or other shell fragments were present. A shelby tube sample from 1.1-1.7 m shows ripple bedding (figure 18). A Shelby tube sample from 2.1-2.7 m contains 0.6 m of horizontally laminated sand (figure 19).

Well sorted, upwarding-fining sand, with horizontal laminations in the lower part, and ripple beds in the upper part, indicate that unit A_1 is a point bar deposit (Reineck and Singh, 1975, p. 231; and Allen, 1965, p. 142).

Unit A_2 . The channel part of the profile is represented by a dark brown, very loose (average N=4), moderately sorted silty sand and sand (tables 3-7, 14 and 15). Unit A_2 contains abundant fresh-water gastropods of two unidentified types. The three holes (4, 6, and 13-14) in which samples were taken showed no regular stratification except for a grayish-brown bed at a depth of about 1.8 m (which probably represents the base of the artifical fill), and shell stratification in hole 4 at 3.6 m.

The position of the unit in the channel, the fine grain size of the sediment, and the abundance of fresh-water gastropods, indicate that the lower part of the unit is channel-fill. From the 1937 aerial photographs and recent topographic maps we estimate that the top 1.8 m of unit A_2 is artifical fill. Also, the height from the ground surface at the site to the bottom of the channel in Heber Dunes park is 2.0 m. Natural filling of the channel probably followed a sequence of events similar to events described by Allen (1965). Channel shift caused by a chute cut-off slowly shifts the stream from

an old to a new channel (Allen, 1965, p. 119). The slow shift allows continued deposition to take place, but under relatively low-energy conditions of overbank flooding. This process allowed the channel to be filled with loose sand and with a minimum amount of silt and clay.

Unit A_3 . Unit A_3 occurs on the east side of the channel at holes 7, 8, 15, and 16 (tables 7, 8, 15, and 16). The unit consists of dark brown, medium dense, moderately sorted sand and silty sand. The sediment is angular and consists of about 95% quartz and feldspar grains. The other 5% of the sediment consists of heavy minerals, mica, and fragments of organic material. The unit is sharply truncated between holes six and seven. A Shelby tube sample from 3 to 3.6 m shows thinly bedded sediment with convolute structures (figure 20). Osterberg piston samples were taken for bulk density measurements near holes 15 and 16. Between 1.83 and 2.59 m we measured the dry density of the overbank silty sand as 1.54 gm/cm^3 . Beween 4.11 and 4.88 mthe dry density was 1.47 gm/cm $_3$. Density samples were also taken in units A_1 and A_2 . Density measurements at A_1 and A_2 were made in the field after the samples had been extruded. The density measurements in units ${\bf A}_1$ and ${\bf A}_2$ are questionable and are not reported. The density samples from A_3 were obtained by cutting the sample tube with a tube cutter, rather than extruding the samples with a piston that compresses the sample.

According to criteria given by Reineck and Singh (1975, p. 245-246) the position of unit A_3 in relation to the channel and unit A_1 , and the grain size distribution of the sediment, indicate that unit A_3 is a natural levee and overbank deposit.

Deposition of the three units (A_1, A_2, A_3) in the upper 5 m was related to the fluvial activity in the relict channel. The relationship between the three units and the channel indicate that the three units are of Late Holocene

age. Because A_2 fills the channel, unit A_2 is younger than either unit A_1 or unit A_3 . Although the three units are approximately the same age, the three fluvial units have distinctly different penetration resistances indicating different relative densities. Relative density may be related to the flow velocity of the stream in the respective environments. The highest relative density is found in the point bar deposit (A_1) . The horizontal laminations in the lower part of the unit indicate deposition from the upper energy flow regime (Reineck and Singh, 1975, p. 10). The low relative-density portion of the point bar deposit contains ripple bedding indicative of the lower energy flow regime. Sand in medium-dense unit A_3 was deposited during overbank flooding. The least dense sediment (A_2) was deposited during low velocity channel filling.

Fluvial and Lacustrine Units

Units B, C, D, E. Alternating beds of silty clay and sand lie beneath units A_1 , A_2 , and A_3 . The silty clays (units B and D) are stiff, reddish-brown and contain very little sand. The uppermost silty clay (unit B) is distinctly graded; silt grades downward to a 10-cm thick layer of alternating 1.2 cm beds of silt and clayey silt, beneath this transition zone lies very fine-grained silty clay. The sands (units C and E) are brown and very dense, and contain fresh-water gastropods. The upper sand unit (C) is well sorted and has sharp contacts with adjacent units, whereas the lower sand (E) is poorly sorted and has a gradational contact with the underlying fine-grained unit.

These alternating sand and clay beds represent channel-fill and lacustrine environments. Sand units C and E are slightly coarser than the channel-fill deposit (unit A_2) in the relict channel. The clay units (B and

D) are typical of sediment laid down in a quiet lacustrine environment. The alternating environments were controlled by channel shifts of the main distributary, the Colorado River (Van De Kamp, 1973, p. 827-848). In the past, natural channel shifts of the Colorado River have inundated the Imperial Valley. During these inundations a lake was formed and lacustrine sediment was deposited. During periods of low lake level the meandering Alamo River deposited shoestring sand beds on the lacustrine sediment. The fluvial and lacustrine sediment is brown and reddish brown indicating that it originated from the Colorado Plateau.

River Park

Tables 18 through 32 contain CPT and SPT data, geologic logs and descriptions, and grain size analyses from drilling sites at River Park. The samples and SPT and CPT data were used to construct the profiles in figure 21 and figure 22. The profiles show three continuous units in the area of the flood plain and the slump. The average penetration resistance and relative density of the units are shown in figure 23.

Flood plain

<u>Unit A.</u> The upper unit (A) consists of loose, brown, sandy silts and clayey silts. The upward change from the clay in unit B to the sandy silt and clayey silt in unit A indicates an increasing energy source, probably a result of the main channel meandering nearer to the east side of the flood plain.

The sandy silts represent a lower flood plain environment, whereas the clayey silts represent a flood basin environment (Helley and others, 1979, p. 35).

Unit B. The middle unit (B) consists of very fine-grained silty clay and clay. In hole I the clay is soft, black, and very organic rich. At hole four

the unit contains two parts; the upper part is firm, has reddish oxidation and yellow concretions, whereas the lower part is softer, grayish-brown and more organic rich. In holes six and nine (within the slump) a very poorly sorted, asphaltic, pebbly silty sand replaces the silty clay. The latter material is artificial fill emplaced about 20 years ago. An Osterberg piston sample 76 cm long and 7.62 cm in diameter was used to take bulk density samples near holes 8 and 15. Near hole 8 we measured the wet density of the clay at 1.77 gm/cm³ between depths of 1.52 and 2.13 m. At hole 15 the measured wet density of the clay was 1.68 gm/cm³ between depths of 3.05 and 3.20 m.

The black, organic-rich clay represents a back swamp environment far from the high energy environment of the main channel (Reineck and Singh, 1975, p. 250, Allen, 1964, p. 168). The clay in hole 1 apparently filled in a depression in the sand. The clay remained soft because it was deposited in a depression and quickly buried (the clay shows no oxidation). Similar sediment in hole 4 was deposited in a more exposed position and became partly desiccated (in hole 4 the upper part of B has reddish oxidation and yellowish concretions).

<u>Unit C.</u> The lowermost unit (C) is a generally dense, well-sorted fine sand. The sand unit appears massive, the only subdivision seen was a color change from brown at the top to grayish brown at the bottom. The sand unit becomes finer away from the river and shows a well defined upward decrease in grain size in holes 8 and 11. An Osterberg piston sampler was used to recover bulk density samples at holes 8 and 15. At hole 8 the measured wet density of the sand, between 2.13 and 2.59 m, was 2.05 gm/cm³. At hole 15 the wet density was measured at 1.93 gm/cm³ between 3.51 and 3.66 m. The contact with the overlying clay is very sharp at hole 8. The CPT soundings show the upper 45 cm of sand to be noticeably less dense than the lower part. This lower

density layer may have been produced by liquefaction and pore-water migration through the sediment during the 1979 and previous earthquakes.

The lateral extent, the thickness, and the grain size distribution of the unit C indicate that the unit is a point bar deposit (Reineck and Singh, 1975, p. 231; Allen, 1965, p. 142).

Slump

The stratigraphy in the off-slump area (holes 8, 11, and 13) is very similar to that of the flood plain. The stratigraphy within the slump (holes 6 and 9) is very different from the stratigraphy of the flood plain. The sediment encountered on the slump included an asphaltic artifical fill. We were informed by a city worker that the area near the slump had been filled about 20 years ago to extend the park. Drilling evidence indicates filling is more extensive in the southern part of the area than in the north where the slump scarp was highest. Profiles of the slump area are shown in figure 22. Data from the holes in the slump area are shown in tables 23 through 32.

Hole 6. The upper 2 m consists of very poorly sorted asphaltic fill. The fill overlies interbedded, dark brown, sand, silt, and clay (unit A) that occurs between 2 m and 3 m. This unit in turn overlies black, sandy silt and clayey silt (unit B). Unit B overlies brown, poorly sorted silty sand (unit C). This hole was only sampled to 4.6 m. It is assumed that unit C becomes coarser and increases in relative density with depth. Units A and B may not be equivalent to A and B on the flood plain. Unit C, however, is an extension of unit C beneath the flood plain. The sediments, of units A and B, in hole 6 are coarser and are more interbedded than the sediments on the flood plain. It is possible that units A and B in hole 6 are actually fill material. In the other hole (hole 9) located on the slump artificial fill rests directly on

top of unit C. A profile of hole 6 is shown in figure 22. The data from hole 6 is shown in table 23.

Hole 8. The stratigraphy in this hole (off the slump) is very similar to that of the flood plain; sandy silt (unit A) overlies clay (unit B) that overlies fine sand (unit C). The sandy silt, clay, and the upper part of the sand are dark brown. The lower part of the sand is grayish brown. This color change is also seen in holes 4 and 11. The overall grain size of the sand decreases upward. A profile of hole 8 is shown in figure 23. The data from hole 8 is shown in table 25.

<u>Hole 9</u>. The sediment in this hole (on the slump) more closely resembles the sediment in hole 6 than that found in the holes on the flood plain. The gravelly asphaltic mixture present in hole 6 is also found in this hole. The presence of the artificial fill indicates that the silty sediment lying above is also artificial. Below the artificial fill is a dense, fine sand that is brown above 4.2 m and grayish below 4.2 m. This sand unit is similar to unit C of the flood plain. A profile of hole 9 is shown in figure 23. The data from hole 9 is shown in table 26.

Hole 11. The sediment in this hole (off the slump) is similar to the sediment in the flood plain. The upper unit in this hole is finer than unit A of the flood plain, and the middle unit is coarser than unit B of the flood plain. The lower unit is similar to unit C of the flood plain. The unit contains an upward fining sand that is brownish above 4 m and grayish brown below 4 m. A profile of hole 11 is shown in figure 23. The data from hole 11 is shown in table 28.

ANALYSIS

Based on the drilling data reported above, an analysis of liquefaction susceptibility was made for units A_1 , A_2 , and A_3 at Heber Road, and units A and C at River Park. The following text is summarized from Youd and Wieczorek (in press) with the analyses updated to include additional data presented in this report.

Heber Road Site

The geotechnical data in figures 13 and 14 were used to estimate liquefaction susceptibility of the soil layer penetrated by the sounding and borings along Heber Road using procedures developed by Seed (1979). Three sand and silty sand units lie in the upper 5 m of the section; a dense, point bar deposit in the west part (unit A_1), a very loose channel fill and artifical fill in the central part (unit A_2), and a medium dense overbank deposit in the east part (unit A_3). Average standard penetration blow counts below 2 m, the depth of the water table, were 4, 11 and 31 in these units, respectively. These blow counts were obtained using the hollow-stem auger technique. Other parameters needed to perform the analyses of liquefaction susceptibility included an average estimated dry density of 1.6 gm per ${\rm cm}^3$, a depth of 4 m to the approximate midpoint of the layer that liquefied, and a maximum horizontal acceleration of 0.6 g (conservatively estimated from strong-motion data published by Porcella and Matthiesen (1979)). The equation given by Seed (1979, p. 210) for calculating cyclic stress ratio, $\frac{\tau_{av}}{\sigma_{a'}}$, is:

$$\frac{\tau_{av}}{\sigma_{o}'} \cong 0.65 \quad a_{max} \left(\frac{\sigma_{o}}{\sigma_{o}'}\right) \quad r_{d} \tag{1}$$

where a_{max} is the maximum acceleration at the ground surface divided by the acceleration of gravity, σ_{o} is the total overburden pressure at the 4 m depth (0.64 kg/cm²), σ_{o}' is the effective overburden pressure (0.42 kg/cm² in this instance) and r_{d} is a stress reduction factor (about 0.95 for a depth of 4 m). Using the above values in equation 1 yields a cyclic stress ratio of 0.56 at the 4-m depth.

To complete the analysis corrected blow counts are required. Curves given by Seed (1979, p. 238) give a correction factor of 1.4 for an effective overburden pressure of 0.42 kg/cm². Applying this factor to the blow counts listed above yields corrected blow counts of 6, 15, and 43 for the channel fill, alluvial deposit, and the point bar deposit, respectively. These data are plotted on figure 24 at a cyclic stress ratio of 0.56. Based on the data in figure 4b we also assumed a correction factor of 2.5 to correct our safety-hammer standard penetration values to values that might be expected with the conventional rope-around-a-cathead technique. These corrected blow counts, for values less than 40, are also plotted on figure 24. Because of the limited data considered, the 2.5 correction factor must be considered as preliminary and very tenuous.

The plot indicates that for a magnitude 6.6 earthquake, the channel fill sand (Unit A_2) is susceptible to liquefaction. The susceptibility of the overbank deposit is marginal, plotting as liquefiable using the hollow-stem auger data and unliquefiable using the corrected data. Data for the point-bar sand (A_1) indicates that deposit is highly resistant to liquefaction. Surface effects at the Heber Road site indicate that widespread liquefaction and large ground displacements did occur in the area underlain by unit A_2 . There was no indication of liquefaction in unit A_3 other than the linear group of sand boils shown in figure 8. These sand boils erupted over a buried drain line

and were likely caused by liquefaction of soil disturbed during construction of the drain. There was no evidence of liquefaction in the area underlain by the point bar deposit (unit A_1). A rise of pore-water pressure could have occurred during the earthquake in either of the latter two units (A_2 and A_3) without leaving evidence in the form of lasting surface effects.

River Park

The data on figures 21 and 22 and the technique described above were used to analyze liquefaction susceptibility of the deposits at River Park. Parameters estimated for that site include a maximum acceleration of 0.2 q (estimated from a strong motion record at Brawley Airport (Porcella and Mathiesen, 1979, p. 25), a dry unit weight of 1.6 gm/cm³, estimated average depths to the mid points of the liquefied layers of 2 m for the silty sand layer and 5 m for the unstratified sand bed, and a water table at the ground surface. From the limited standard penetration data at that site, a value of 3 blows per foot is estimated for the silty sand and a average of 7 blows per foot for the upper part of the unstratified sand. Corrected blow counts for these layers are about 4 and 10 blows per foot, respectively. The calculated stress ratio for both layers is 0.25. Corrected blow counts are plotted against this value in figure 24. Estimates of the conventional standard penetration values are also plotted using a 2.5 correction factor. The plot shows that the upper layer of silty sand (unit A) is susceptible to liquefaction. Data from the looser part of the unstratified sand (unit C) plot as susceptible for the unadjusted hollow-stem auger blow counts and nonsusceptible for blow-counts adjusted to the conventional technique using the tenuous correction factor of 2.5. As noted above, textures of the sand

boil deposits at River Park indicate that liquefaction did occur in both the silty sand (unit A) and the unstratified sand layers (unit C).

The slump on the New River bank near Brawley originated from liquefaction of fine sand. The best developed surface features on the slump are at the north end where the vertical displacement is 1.5 m and a dome-like toe is developed in the river channel. The low penetration resistance in the upper part of unit C and the very poor sample recovery of the same material indicate that the low density sand is liquefiable. The shape of the slump and the low density of the underlying sand indicate that liquefaction occurred in the sand. The overlying artifical fill and the fine sediment settled and rotated into the weakened sand.

SUMMARY

Investigative Technique

The cone penetration test served as the primary tool for defining the geometry of sediment units and for estimating the density and strength of soils penetrated. The profile developed from CPT data was completed in a much shorter time than could have been done by conventional drilling and sampling methods and shows the stratification in greater detail. The main disadvantage of the CPT is that samples are not recovered. Samples and N vaules for evaluating liquefaction susceptibility were obtained from standard penetration tests. Use of the SPT in conjuction with the CPT allowed fewer but more optimally positioned SPT to be made while gaining a detailed profile. To fully sample the site under investigation, continuous, disturbed samples were taken by the cork-screw technique. Sedimentary structures are disturbed in this procedure but large amounts of material are recovered for testing.

Heber Road Site

In recent geologic time a lacustrine environment existed in the Imperial Valley due to diversions of the Colorado River and the formation of ancient Lake Cahuilla. In the central part of the lake fine-grained clays were deposited. As lake level rose and fell due to Colorado River diversions and lake evaporation, lacustrine clays were interbedded with sands deposited by meandering channels such as the Alamo River. Superimposed on the alternating lacustrine and fluvial deposits was a sequence of deltaic sands deposited at the mouth of a stream entering the lake. One such delta formed at the mouth of an ancient stream 1 to 2 km west of the present course of the Alamo River. A remnant channel marks the former course of that stream. Liquefaction and a lateral spread developed within the channel at the latitude of Heber Road where we conducted subsurface investigations.

The geology at the Heber Road site consists of; (1) fluvial sediment made up of, point bar, channel fill, and levee deposits, and (2) at least two cycles of channel sands alternating with lacustrine clay. The fluvial deposits lie within 5 m of the surface and display a wide range of geotechnical properties. The point bar deposit (unit A₁) contains medium dense to dense, thinly bedded very fine sand. An analysis showed that high density of the sand precludes any liquefaction with the accelerations accompanying the October, 1979 earthquake. This conclusion was supported by field evidence that showed no sand boils or permanent ground displacements occurred in the area of the point bar deposit. A rise of pore-water pressure could have occured in the sand during the earthquake without leaving surface evidence of the increase.

The channel fill (unit A_2) contains very loose very fine sand. Our analysis shows that this very loose channel fill is susceptible to liquefaction. A lateral spread, ground cracks, and sand boils formed in this area during the earthquake confirming that liquefaction did occur in that unit during the October 1979 earthquake.

The overbank deposit (unit A_3) contains medium dense fine sand. Our analyses indicate that this sand is either susceptible or marginally susceptible to liquefaction, depending on whether blow count adjustments to correct the data to conventional SPT values are valid. Except for a linear group of sand boils that erupted over a burried drain line, no ground displacements or sand boils were found on the overbank deposit (unit A_3). However, a pore-water presure rise may have occurred within the deposit.

The sand below a depth of 5 m is dense enough to resist liquefaction under the conditions created by the October, 1979 earthquake.

River Park Site

The three sediment units at River Park lie beneath the New River flood plain. These units represent a range of fluvial environments. The lowermost unit (C) is a medium dense fine sand typical of a high energy point bar deposit. The upper 0.5 m of this sand is loose, probably as a consequence of liquefaction during this and past seismic events. The middle unit (B) is a silty clay typical of a very low energy backswamp deposit. The uppermost unit (A) is a very loose silty sand and clayey silt typical of a moderate energy flood plain and flood basin deposit. From penetration tests the point bar deposit was found to be medium dense, the backswamp clay very soft, and the flood plain sediment, soft/loose. The uppermost part of the point bar was found to be significantly less dense, especially near the river. An analysis

of liquefaction susceptibility was performed on the silty sand in the upper layer (A) and on the upper part of the fine-grained sand (unit C). The analysis showed that both units should have liquefied. Evidence that the units did liquefy include; sand boil deposits on the flood plain with two distinct grain-size distributions. Some sand boils had grain size distributions similar to the silty sands in unit A. Other sand boils had grain-size distributions similar to the fine sand in unit C.

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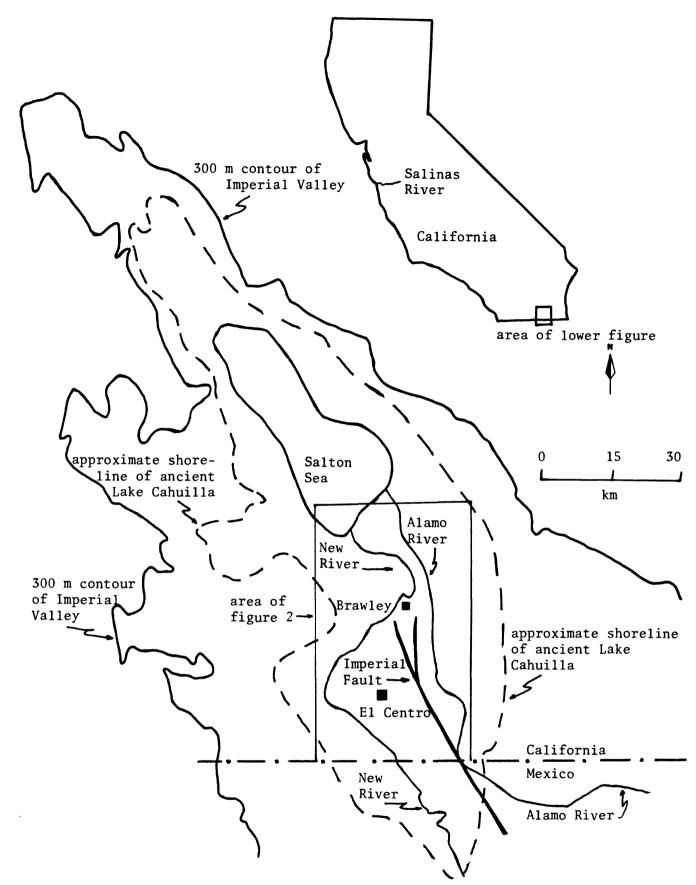


Figure 1. Imperial Valley, California, with valley outlined by the 300 m topographic contour. Extent of ancient Lake Cahuilla marked with dotted line (figure adapted from Van De Kamp, 1973, p. 830).

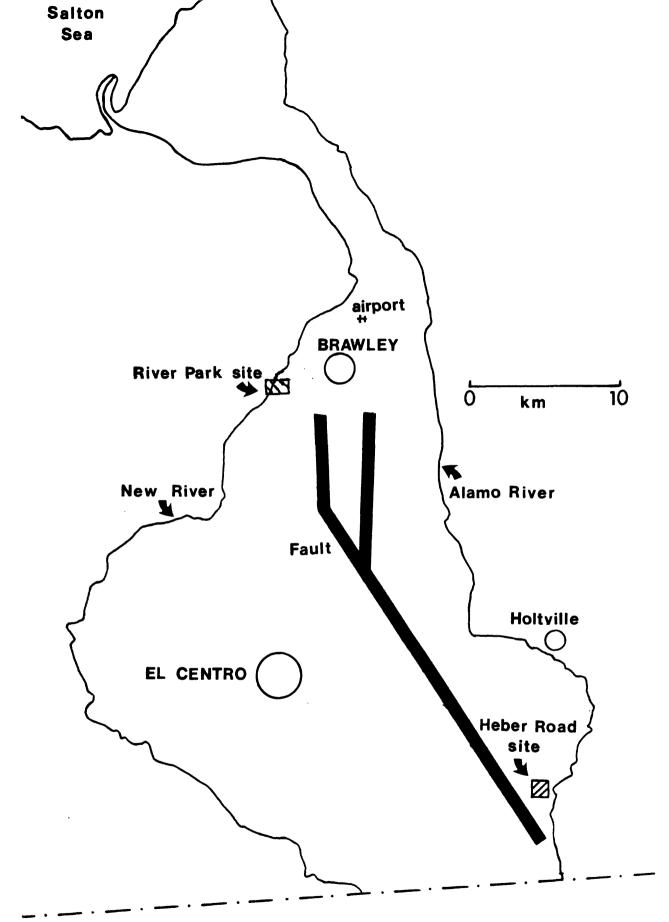


Figure 2. Southern Imperial Valley with Heber Road and River Park sites.

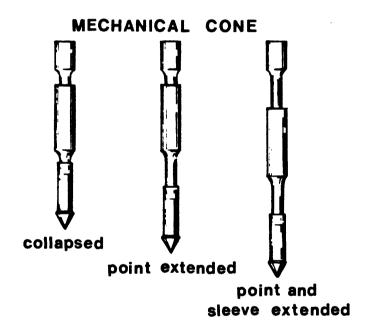
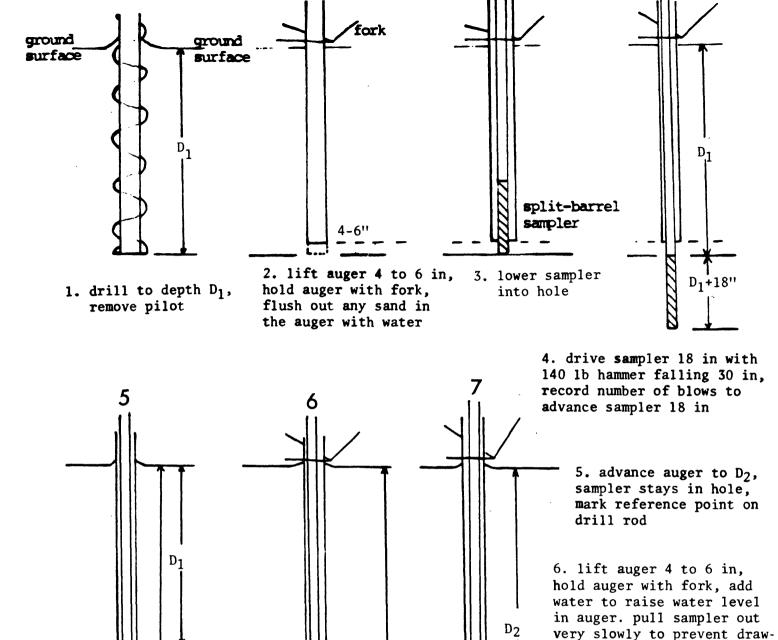


Figure 3. Mechanical, Begemann-type Dutch cone penetrometer. The penetrometer is shown in its three stage operation; 1) collasped, penetrometer is ready to advance to new testing depth, 2) point extended, point resistance is measured, 3) point and sleeve extended, side friction is measured (figure from Sanglerat, 1972, p. 9).



ing sand and water in auger.

7. lower clean sampler to bottom of hole, check reference mark, if more than 3 in difference, flush hol if more than 6 in differ-

ence advance auger 2 ft. deeper and flush hole with water, drive sampler as noted in step 3 and repeat

 $\frac{1}{4-6}$ " steps 4 through 6.

Figure 4a. Procedures for standard penetration test (SPT).

D1+18"

 D_2

 D_2

4-6"

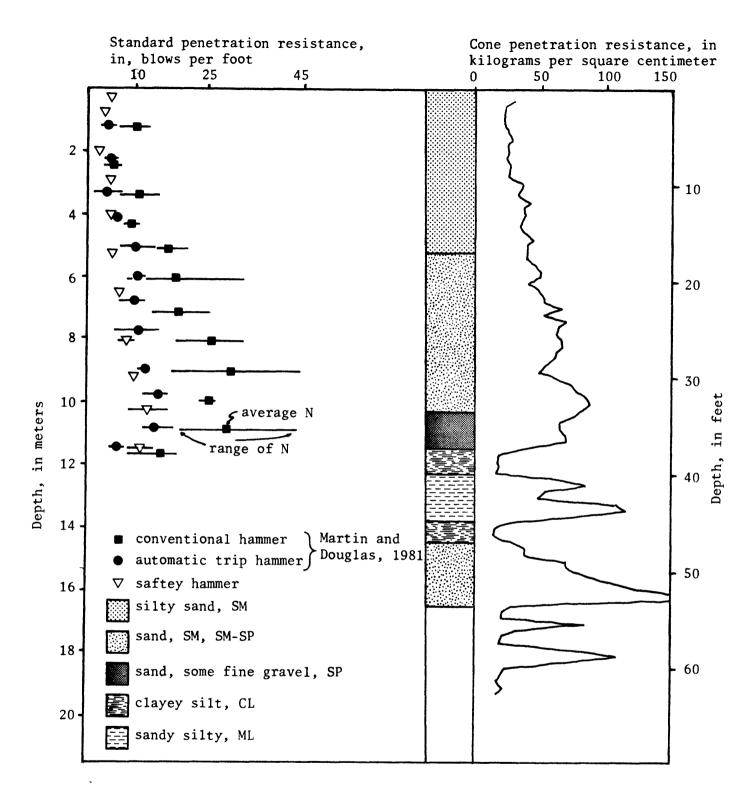
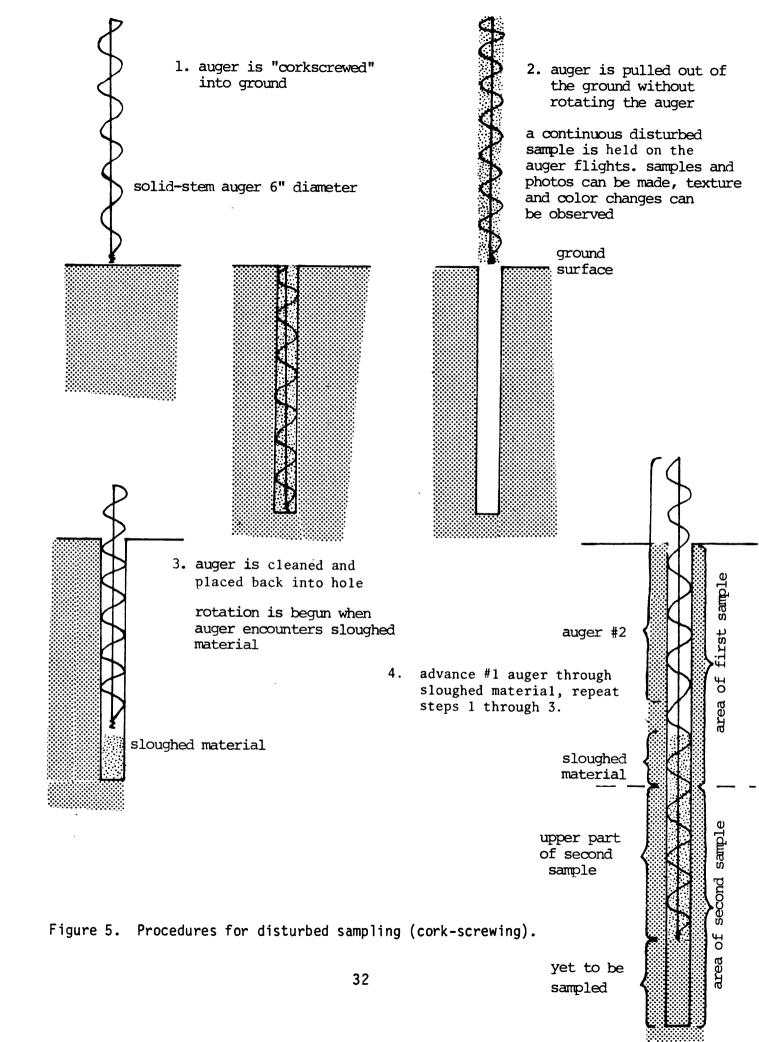


Figure 4b. Standard penetration tests made with 3 different hammers are compared to each other and to a cone penetration test (average of 3 tests) at a site near the Salinas River, California. The 3 hammers used were; 1) conventional hammer with cathead-winch (4 tests), 2) automatic trip hammer, used with a cathead-winch, that controls the fall of the hammer (4 tests), 3) safety hammer with free-fall winch (2 tests).



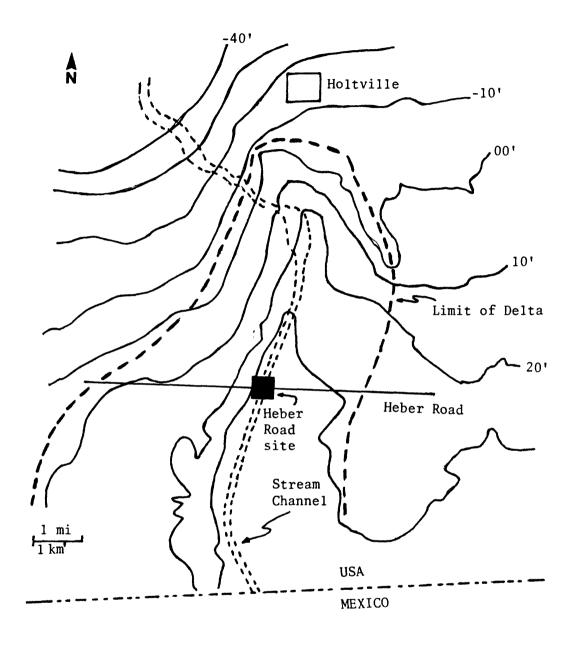


Figure 6. The Heber Road site is located across the relict stream channel that cuts through an ancient delta. The delta is defined by topographic contours and by sandy soils identified by Strahorn and others (1924). Sand boils, ground cracks, and a lateral spread were concentrated within the relict stream channel (figure from Youd and Wieczorek, in press).

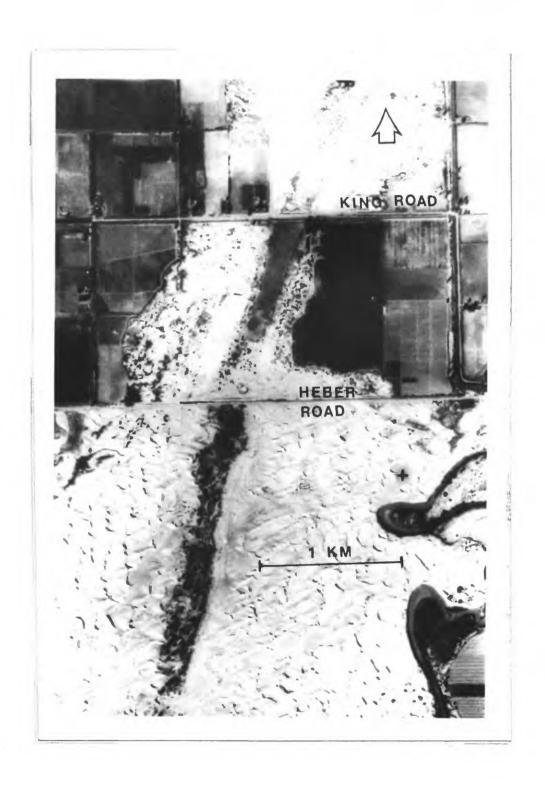


Figure 7. Aerial photograph from 1937 shows outline of relict channel (figure from Youd and Wieczorek, in press).

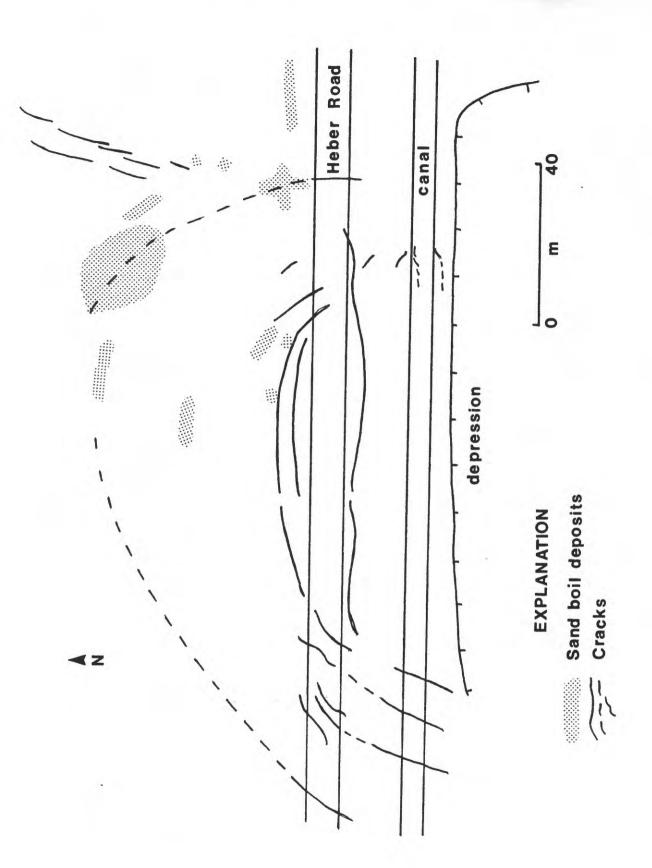
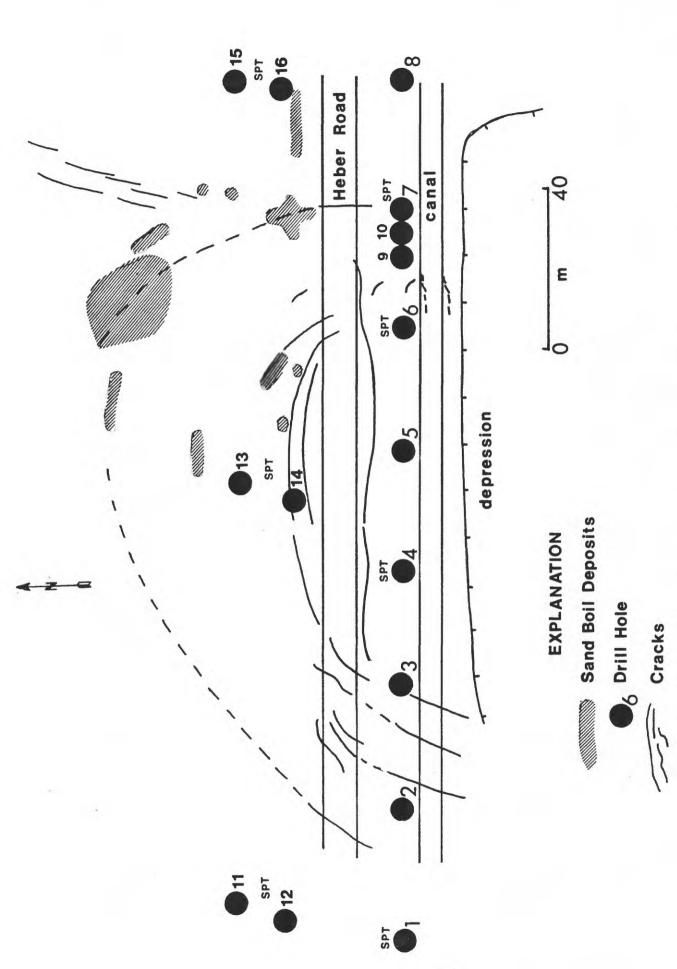


Figure 8. Location of sand boils and ground cracks at Heber Road site (figure adapted from Youd and Wieczorek, in press).



Figure 9. Location of cone penetration tests at Heber Road (view looking west). The bend in the canal was caused by southward movement of the lateral spread.



site. Cone penetration tests were tests (SPT) were made at holes 1, and 14, 15 and 16. Bulk, disturbed Tube samples were taken at holes 1 4, 6, 7, and between holes 11 and 12, 13 and 14, 15 and 16. samples were taken at holes 1, 4, and 7. Tube samples and 7, and near holes 11 and 12, 13 and 14, 15 and 16. made at holes 1-16. Standard penetration tests Figure 10. Location of tests at Heber Road site.



Figure 11. View of Heber Road site looking south. The relict channel is outlined by the solid lines, the general shape of the lateral spread is marked by the dashed line.



Figure 12. Numbers mark the location of holes on the flood plain at River Park (view looking west).

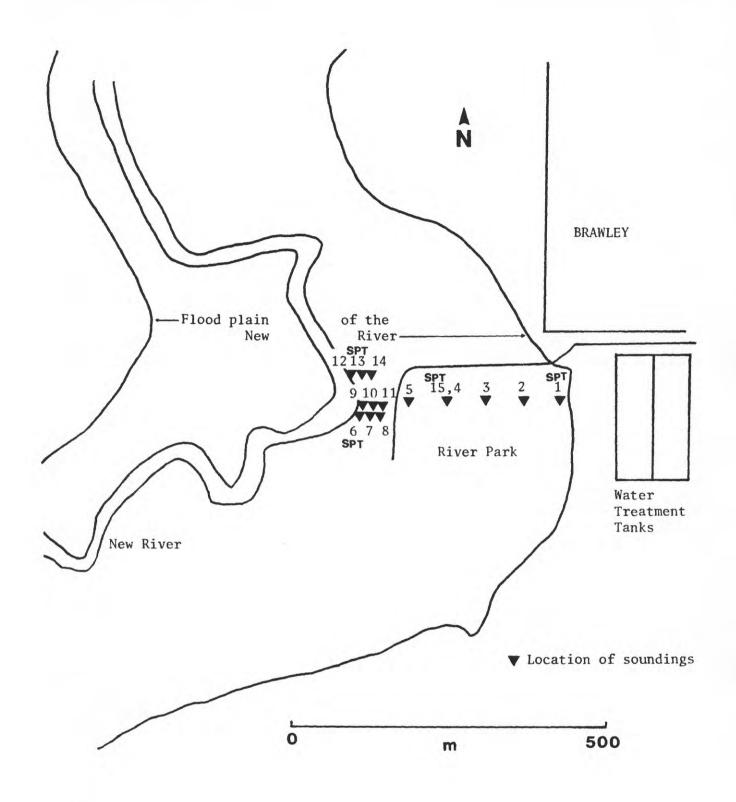
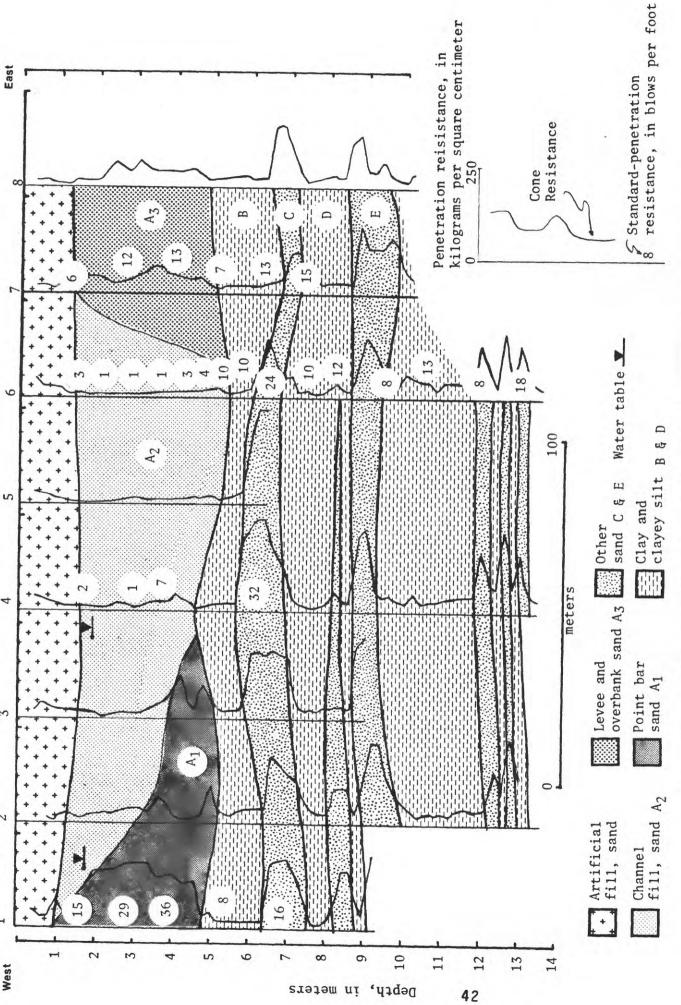


Figure 13. Location of tests at River Park site. Cone penetration tests were made at holes 1-15. Standard penetration tests (SPT) were made at holes 1, 6, 13, and 15. Disturbed samples were taken at holes 1, 4, 6, 8, 9, and 11.



Figure 14. View of River Park looking east. Location of holes is marked by numbers. Profiles of holes 1-5 are shown in figure 20. Profiles of holes 6-14 are shown in figure 21.



diments at Heber Road site. Unit A₂ represents loose and A₃ represent point bar deposits and overbank Units B-F represent alternating stream channel and Profile of sediments at Heber Road site. channel fill. Units A₁ deposits, respectivly. lacustrine deposits. Figure 15.

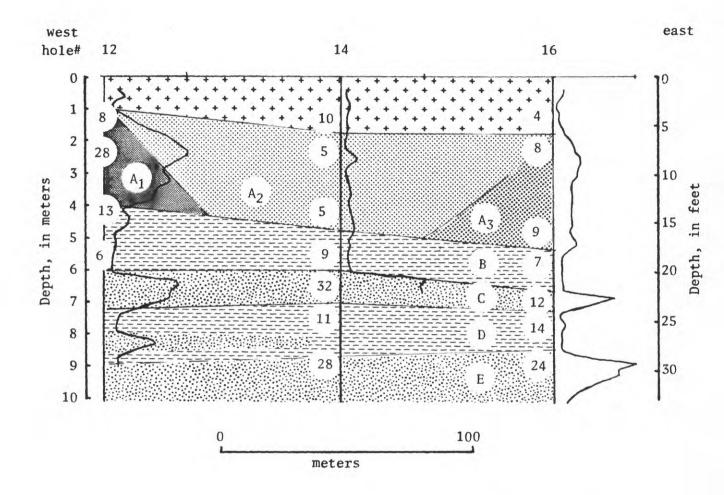


Figure 16. Profile of sediment on north side of Heber Road.

UNITS	N	\bar{q}_c	$\bar{\mathtt{R}}_{\mathbf{f}}$	$\overline{\mathtt{D}}_{\mathbf{r}}$	approximate
Fill	5	22	3.0	52%	depth, in meters
A ₁ - top * fine sand	12	75	3.40	80%	1.5
A ₁ - bottom * fine sand	31	160	2.87	119%	5
A ₂ - fine sand	4	20	2.76	23%	5
A ₃ - fine sand	11	49	2.46	69%	5
B- clay	8	22	3.36	-	6
C- fine sand	23	169	2.56	105%	7
D- clay	13	27	4.00	-	9
E- fine sand	26	138	2.53	88%	10
F- clayey silt	11	38	3.17	-	12

N = Average N, in blows per foot

gc= Average cone resistance, in kg/cm²

Rf= Average ratio, in percent

D_r= Average relative density, in per cent
Mechanical cone values converted to
electrical cone values (Schmertmann, 1978a, p. 59)
Relative density was calculated from:
D_r = 1/2.91*ln(q_c/12.31*d_c'0.71)*100%

σ_v'= effective vertical stress (from Schmertmann, 1978a, p. 40)

* The top part of unit A₁ is characterized by ripple bedding and medium dense sand. The bottom part of the unit is characterized by horizontal bedding and dense sand.

Figure 17. Generalized characteristics of sediment at Heber Road site. Data include average; blows per foot (N), point resistance/friction ratio, and relative density (D_r) .

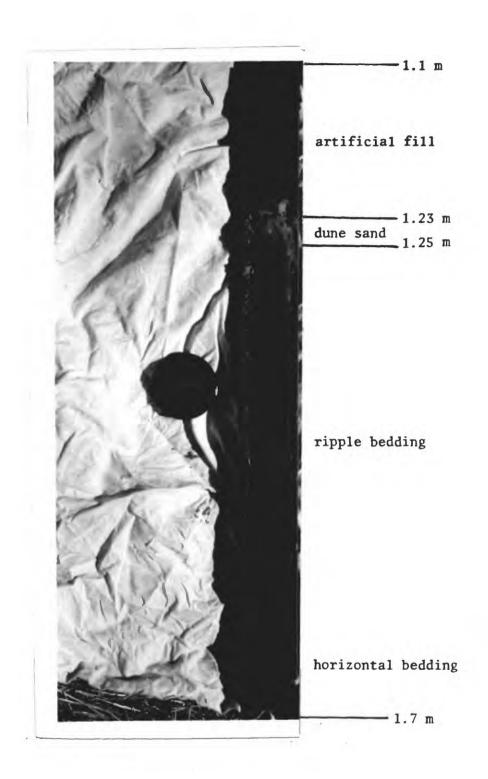


Figure 18. Standard penetration sample, 1.1-1.7 m, hole 1. 0-13 cm, artifical fill; 13-15 cm, thin layer of dune sand (?); 15-51 cm, ripple bedding from the upper portion of the point bar deposit. The change to horizontal bedding can be seen at the bottom of the sample.

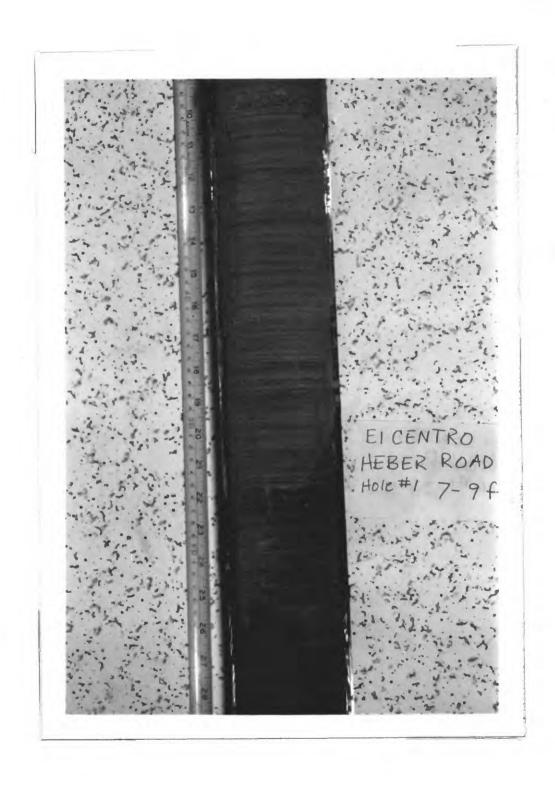


Figure 19. Tube sample, 2.1-2.7 m, hole 1. This sample contains 61 cm of undisturbed horizontal bedding from the point bar deposit.

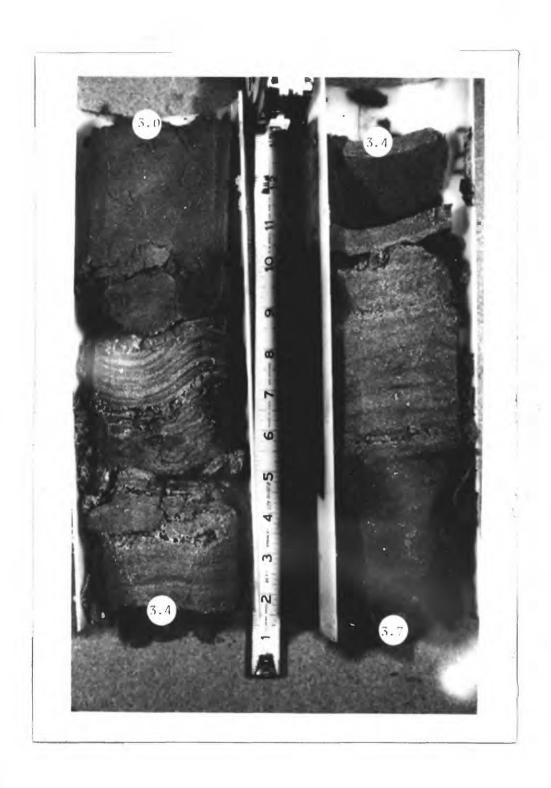
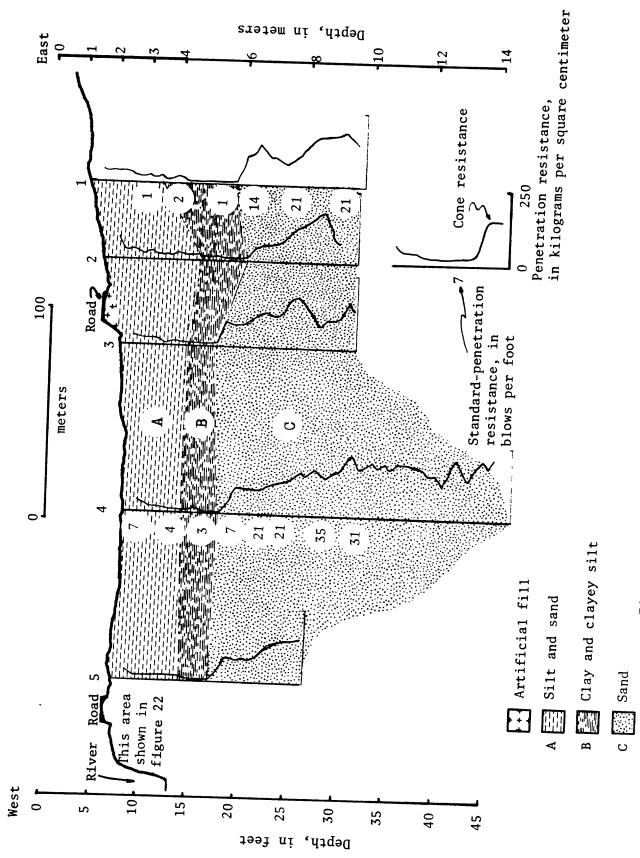


Figure 20. Tube sample from the overbank deposit, 3.0-3.7 m, hole 7. The left hand sample is from 3.0-3.4 m, the right hand sample is from 3.4-3.7 m. The disturbed bedding at 3.2 m is probably from sampling disturbance.



represents overbank flood plain deposition. Unit B represents flood basin deposition. Unit C represents channel deposition, probably a point bar. Figure 21. Profile of sediment units on River Park flood plain. Unit A

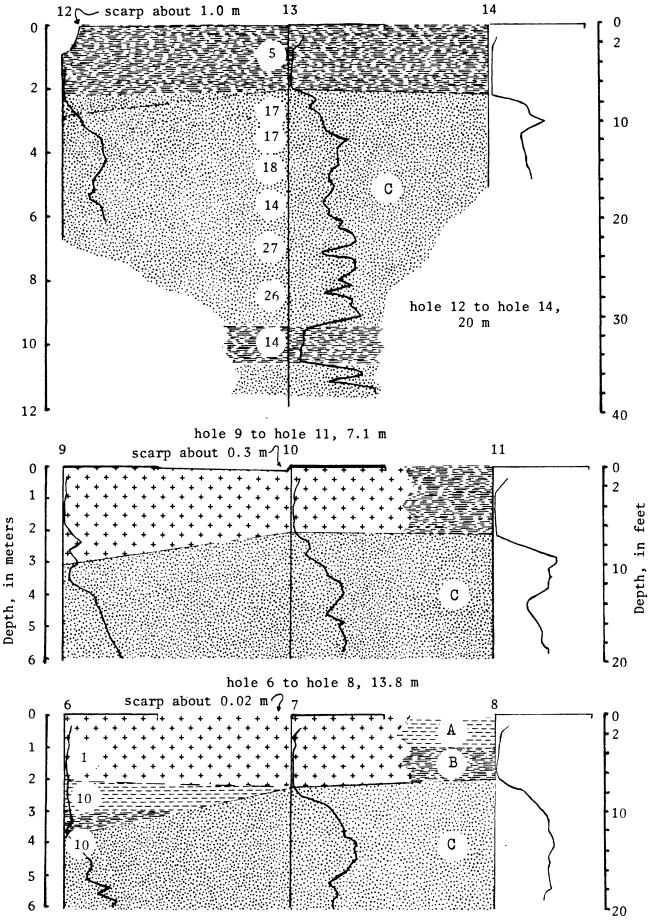


Figure 22. Profile of sediment units near slump area. In general, the units carry over from the flood plain. The most noticeable difference is the artificial fill near holes 6, 7, 9, and 10.

UNITS	N	٩̄c	$\bar{R}_{\mathbf{f}}$	D _r	approximate
Fill*	-	18	2.13	-	depth, in meters variable
A- clayey silt to silty sand	3	24	2.57	54%	2.0
B- clay to clayey silt	3	9	3.40	-	3.0
C top** fine to medium sand	7	69	2.28	80%	
C bottom** fine to medium sand	23	138	2.51	102%	
C all** fine to medium sand	21	117	2.41	99%	11.0

N = Average N, in blows per foot

$$D_r = 1/2.91*ln(q_c/12.31* \sigma_{v'}^{0.71})*100%$$

Figure 23. Generalized characteristics of sediment at River Park site. Data include average; blows per foot (N), point resistance/friction ratio, and relative density (D_r) .

 $[\]bar{q}_{c}$ = Average cone resistance, in kg/cm²

R_f= Average ratio, in per cent

 $[\]bar{D}_r$ = Average relative density, in per cent Mechanical cone values converted to electrical cone values using Schmertmann, 1978a, p. 59. Relative density was calculated from:

^{*} Fill replaces units A and B in holes 6 and 9 on the slump

^{**} The top 1-m of unit C is medium dense sand, whereas the bottom of the unit is dense sand

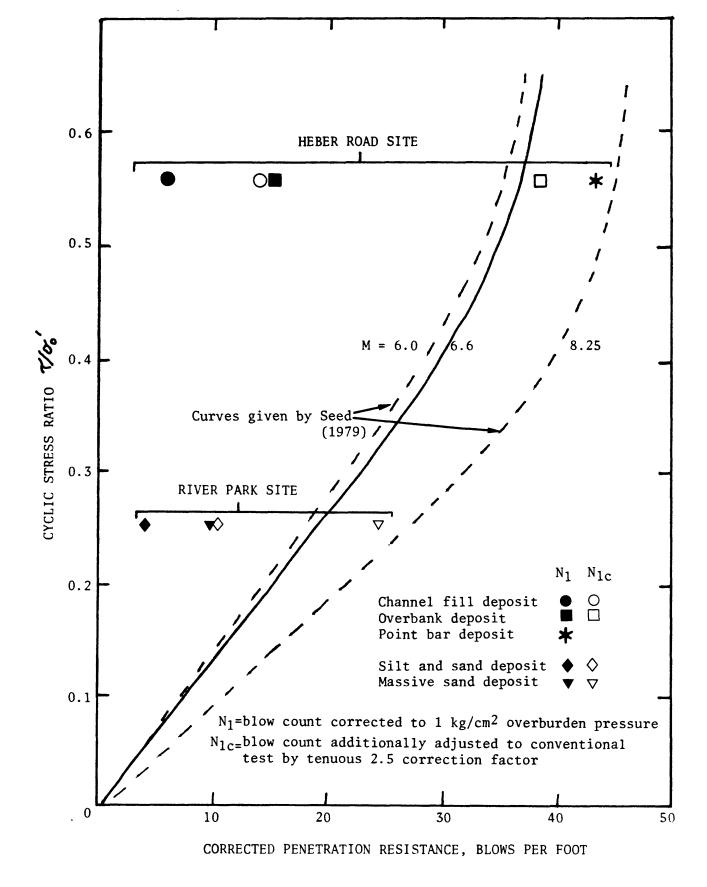
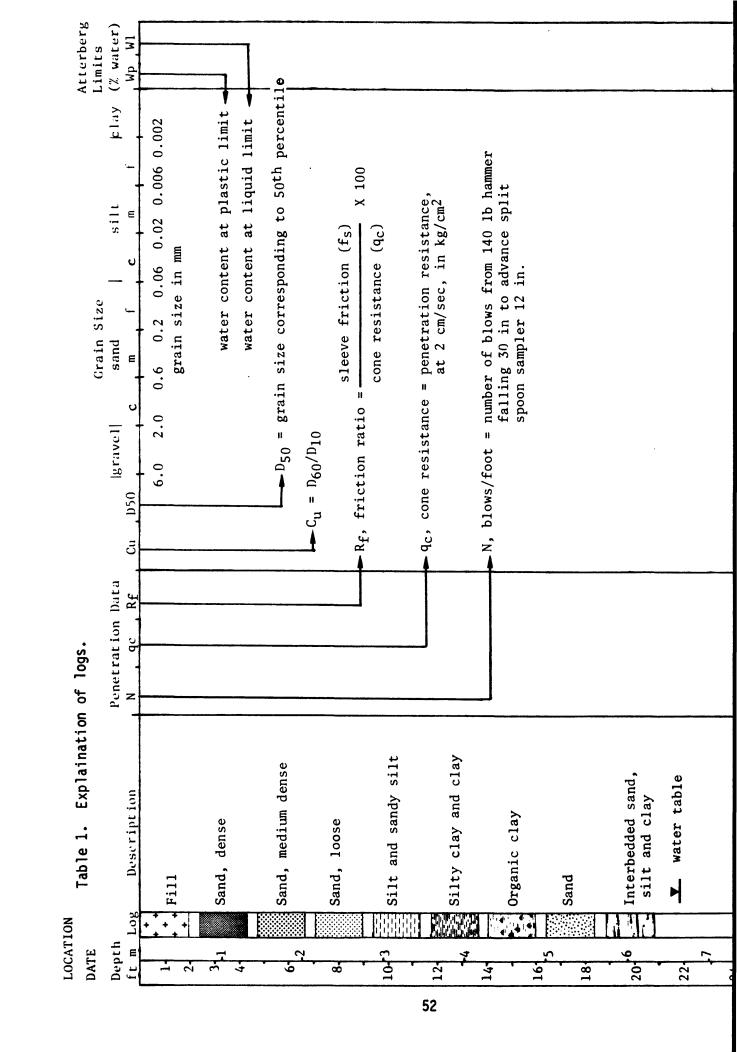


Figure 24. Liquefaction susceptibility from cyclic stress ratio and the corrected penetration resistance, following the procedure developed by Seed (1979).



LOCATION EL CENTRO- Table 2. Log of Heber Road 1.

pth		ember 1979	Penetra				. D50	gravel		Grain sand		1	silt	£	þ:1ay		at
m	Log +	Description	N .	qc	Rf	Cu	υ50		с	, m	<u>f</u>	c	<u>, m</u>	f		Wp	
1	* . *	fill, fine sand	5	52	1.8												
1	[+]	,		32 32	3.3 2.5											İ	
1	+ }		Ι ε	38	2.1	2.8	.100			1	78	15	2	1	3		_
[]				36	5.6												
				70 34	2.7 4.0												
		<u>.</u>	13	30	2.1	2.7	.105	-	-	1	85	10	4				
-2		palino	16		2.7	2.4	.110			1		13	2			-	
		(CD) fine and lead	16		3.2	2.4	.100		_	1	80	13		20			
		(SP) fine sand, dark brown (7:5YR4/4), dense	16	50	3.2 2.7	2.4	.100	-	-	-	ου		•	20			
		well sorted, abundant	29 18 - 18		2.2	2 7	117			2	0.5		2			1	
-3		horizontal laminations	17	76	2.3	2.3	113			.2	85,	11_	2			1	
			17	76	3.6	2.0	.115	-	-	3	87			10			
			14 - 14		4.4 2.5	2.3	.116	-	-	5	84		:	11			
			36 14	8	2.3	1 8	120			4	0.0			0			
.4				8	4.9					4	88			-8			_
	, î		11 13		2.9 2.7	2.2	.120	_	_	6	84		,	10			
		(CI_MI) =:14	3	56	3.0		•120	_	_	Ū	04			10		1	
5		(CL-ML) silt, dark brown (7.5YR4/3)		8	5.0	<u> </u>										1_	
1		very poorly sorted		4	2.8	-	.021	-	-	-	8	43	24	9	16	20	2
		(CL) silty clay,		!4 !2	2.8 3.6												
		reddish brown (5YR4/3)	2	20	4.0												
6		reddish mottling,		24	4.2	<u> </u>	.003				4	9	27	22	38	25	_
		medium			5.0 11.1	l											
		(SP) sand, fine, med	18	30	0.4												
_		dense, mod-well sorted	- 19 16 19	10 10	2.1 2.5												
7		contains turret	16		1.3	3.8	.130			1 F	65					+	
		gastropods, dilatant	10	4	3.3	3.0	•130	-	-	15	05		2	20			
		(CH) clay, reddish		8 4	4.4 3.8												
.8		brown (5YR4/3) medium	2	0 :	25.3											.	
~	¥.		4 16	4	6.1	-	.001	-	-	-	-	1	13	25	62	28	6
		(SP) sand, fine, dark brown (7.5YR4/4)	16	0	2.0 1.7	3.8	.135	_	_	15	71	6	2	2	4		
1		_well sorted clayey silt	8	0	3.8		,			= =		-	-	_	•		
9		clayey silt			24.4											-	
ı		Sanu. delise	20 26	0	2.7	3.3	.140	-	-	13	73	11	3				
ĺ			20	-													
10		 													-		
				_												1	
11															-		-
ļ																	
12																-	
ļ																	
1																	
ŀ																-	
13																	
13																	
13							5 0										
							53										
13							53	- .			<u>.</u> .						

LOCATION EL CENTRO- Table 3. Log of Heber Road 2.

DATE Dece	Table 3	3. Log	of He	eber Roa	d 2.									Atterber
Depth				n Data			gravel	sar	ln Size nd	1	silt		þlay	Limits (% water
ft m Log	Description	1 N		Rf	Cu	D 50	с	, m	f	, c	m	f		Wp W1
2-			24	0.0										
3,1			16 10	2.5 6.7										
4-			30 24 16	3.6 2.8 3.3 2.7										-
			20	3.3 2.7										
6 2			32 34	3.5										
8-			24 42	2.8 1.9										
			44 48	1.8 1.7										
10-3			36 40	3.3 3.3										
12			80 58	2.8										
+4			30 12	3.6 6.7										
14-			10	2.7										
			20	2.0										
16 5			40 24	6.3									<u></u>	
18-			24 36 36	5.6 2.2										
				3.7 4.4										
20 6			36 34	27.0 3.9										
22-			180 154	1.7 3.0 3.3										
			130 116	2.4										
24-			66 3 6	4.0 4.4										
26-8			32 24	3.3										
°			88	3.3 1.8 1.4										
28-			92 96 22 128	3.8 15.8 1.6										
9				1.6 2.6										-
			210 116 68 50	2.6 3.3 2.8 3.3 3.2										
32			6 8 5 0	3.3 3.2										
10			3 6 18	2.6			1							
34-			30 24	2.7										
36-11			30	2.7										
			20 42 48 30	0.3										
384			30 40	3.1										
40-12			50	0.8										
			150 76 240 116	2.6 4.4 2.7 3.9 2.7 11.3 0.3 4.2 3.1 4.0 0.8 3.3 5.6										
13			116	۷.0										
44-														
						į	54							
46-14														
48-														

Table 4. Log of Heber Road 3.

Atterber DATE December 1979 Grain Size Limits Penetration Data silt clay (% water Depth gravel sand ft m Log Description $R_{\mathbf{f}}$ Cu D50 Wp W1 qc m С m 2.2 24 1.5 44 2-2.4 3.3 2.6 2.0 34 24 26 20 16 0.8 14 3.8 16 1.7 8 14 1.3 3.7 3.3 20 18 20 20 10+3 1.3 2.7 3.0 30 44 12-4.3 2.5 50 84 2.0 7.8 4.1 124 46 26 92 52 0.9 3.8 16 22 22 3.0 18 20 4.0 26 5.1 1.1 180 2.2 20 180 170 2.5 184 22 170 2.7 5.9 2.7 160 36 20 24 2.4 28 22 1.8 36 28 38 26 8 0.5 3.9 18 29.6 28 200 220 30-32 34. 36+11 38-40 42 44. 55 46-14 48

Table 5. Log of Heber Road 4.

Atterberg DATE December 1979 Grain Size Limits Depth Penetration Data gravel silt clay (% water) sand ft m Log Description Cu D50 qc Wp Wl 40 2.0 1. .100 8 62 12 4 10 32 2.1 2fill, fine sand 26 2.6 20 3-080 12 4.4 12 1.1 brownish 2 18 .110 8 67 10 14 1.0 6 3 6 👤 grayish 12 1.1 _ dark grayish brown 6-10 1.3 .2 71 10 dark brown 10 4.0 14 1.9 (SM) silty sand and 8. 3.7 .110 2 1 6 74 13 4 24 1.1 very fine sand, brown, 18 2.2 very loose, moderately-6.6 .100 70 3 3 16 4 8 poorly sorted 10+3 2.0 20 28 2.4 .020 4 38 8 6 13 31 28 3.3 11 13 12 28 1.9 .112 63 12 4 4 4 46 1.4 140 50 (SM) sand, poorly sorted 34 1.2 14-28 4.8 .017 2 5 38 26 11 18 18 29 8 1.7 .001 2 (CL) clayey silt and 18 18 16 58 6 **3.**7 silty clay, reddish 16 16 brown (5YR4/3) with 18 3.0 reddish mottling 22 4.8 redder below 17.5' 18 18 14.8 180 3.0 (SP) fine sand, brown 220 2.4 well sorted, dilatant 20 280 2.6 240 2.8 32 4.1 .130 13 76 2 2 silty sand, poorly 1 6 160 3.2 sorted at base 22 124 3.2 .082 6 5 61 16 4 8 40 <u>6.0</u> 20 2.7 .002 15 23 49 24 49 (CL) silty clay, reddish 12 24 4.4 brown (5YR4/3), medium 12 2.2 stiff, contains interbeds 14 6.7 of silty sand 20 8.7 2618 (SM) silty sand, very 1.3 6.7 60 .100 63 6 14 poorly sorted 20 18 28 33.3 (SM) fine sand, poorly 200 1.3 220 sorted 2.4 .150 66 60 4.4 30-3.3 2.7 36 (SM) silty sand, very .090 9 7 5 53 13 13 40 poorly sorted 28 5.7 32 20 clayey silt 0.7 10 4.9 46 .090 4 59 5 7 6 19 22 3.6 34 clayey silt 24 4.4 24 5.0 2.1 38 36+11 34 3.9 3.0 3.2 36 42 38-2.9 46 160 ł12 76 5.3 40-58 1.1 220 2.6 54 3.7 42-2.7 160 13 50 2.7 34 2.4 44 30 2.7 30 4.4 56 46+14 110 48-

EL CENTRO-LOCATION

Table 6. Log of Heber Road 5.

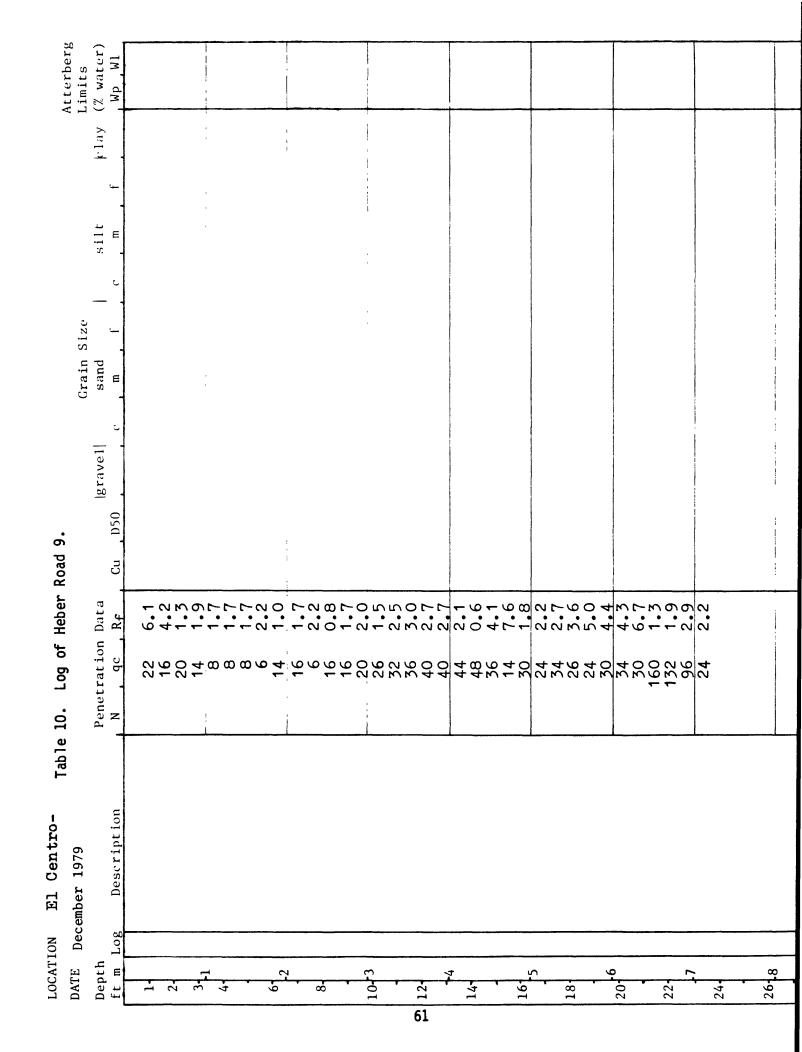
Atterberg DATE December 1979 Grain Size Limits Depth Penetration Data gravel sand 1 silt clay (% water) ft m Log Description Cu D50 f qc R_f С Wp W1 m m 32 28 22 1.7 1-2-1.2 3.8 5.3 2.2 14 3-10 6 6 6 4.4 2.2 2.7 6 10 3.3 6.7 8 4 8-14 1.0 1.3 2.0 3.3 2.7 20 20 10+3 12 10 14 1.9 12 1.9 1.7 14 16 1.7 1.8 2.9 2.2 3.3 16 22 14 24 16 5.3 1.3 14.3 6.7 2.2 10 20 18 28 40 180 280 1.9 20 260 22-24. 26 8 28-30 32 10 34-36+11 38 12 40-42-13 44. 57 46+14 48-

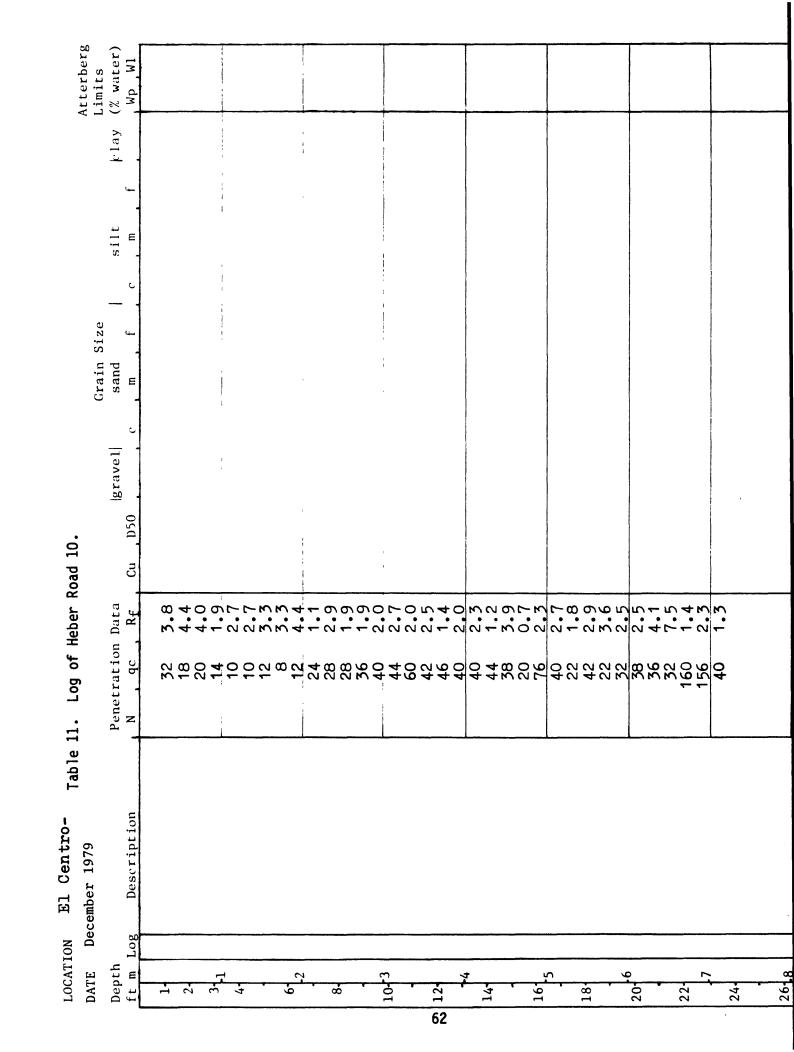
Table 7. Log of Heber Road 6. Atterber, DATE December 1979 Limits Grain Size Depth Penetration Data gravel sand silt clay (% water Cu D50 ft m Log Description . Rf Wp W1 qc 50 2.7 28 2.9 26 5.1 fill, fine sand 28 12 4.4 10 4.0 1.9 14 .110 q 67 12 6.7 61 12 -2 grayish 12 2.2 brownish 10 4.0 8-3.3 8 10 4.0 1 (SM) very fine-fine .120 2.7 9 77 14 14 1.9 sand, dark brown to 1013 reddish brown, very 10 4.0 2.9 14 loose, dilatant, 12 moderately sorted, 12 2.5 16 abundant gastropod 18 2.6 shells .118 3.3 3.3 2.2 16 14 20 24 4 22 1.8 1.8 30 16 20 4.7 10 3.1 25 .132 61 14 10 2.7 16 2.5 18 silty clay, dark redd-22 2.4 10 ish brown (5YR3/4)stiff 28 19.0 20 (SP) fine sand, brown, 80 3.3 medium dense, well 24 160 4.0 1.6 .150 82 18 sorted, gastropods 104 4.1 22. 3.9 3.3 48 10 60 002 3.8 3.3 14 silty clay, dark 24 20 reddish brown (5YR3/3), 6.7 10 stiff 12 2.2 28 1.0 2618 40 2.0 12 .003 7 23 23 44 16 3.3 6.7 20 28 100 2.7 sand, dense, 6 ft of sand flowed into auger 160 1.3 128 2.5 30 no samples 64 5.4 .025 3 38 10 9 13 27 28 2.930 4.4 (ML) silt, clayey silt, and sand, reddish brown (5YR4/3.5), stiff 32 0.2 64 10 50 9.3 44 0.3 moderately-poorly 34 6.7 48 sorted, interbedded, 13 20 12.5 4.0 .020 2 48 33 11 beds 2-7 in thick 46 36+11 5.8 3.0 46 40 38 3.9 38-34 3.9 (SM) very fine sand .088 73 5 144 11 dark brown, gastropods 76 6.0 40 (SM) sand and clayey 30 6.7 silt, clayey silt is 180 3.5 reddish brown (5YR4/3.5) 46 0.3 sand is dark brown (7.5 42 150 YR4/4), sand is dense 13 4.0 .005 18 76 12 1 16 19 13 39 clayey silt is stiff 34 22 3.9 44 10.9 76 2.6 90 46+14 58 a, first SPT b, second SPT 48

Table 8. Log of Heber Road 7

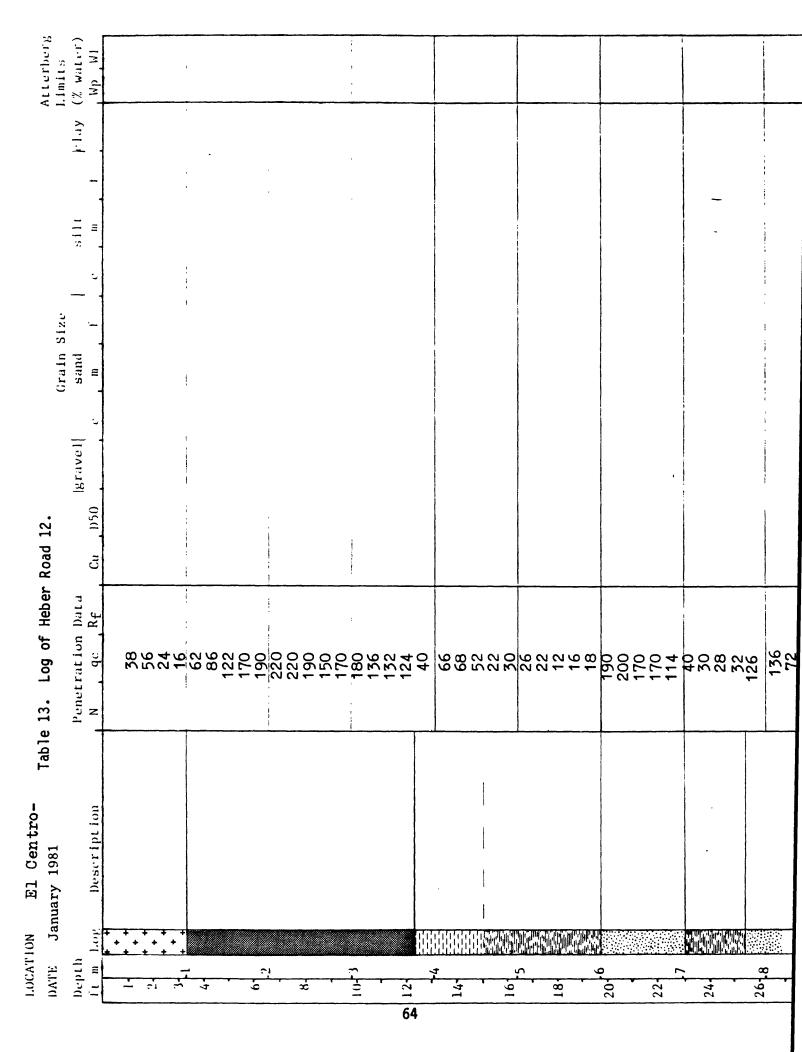
	ATE		Table ember 1979	8.	Log of	Heber I	Road 7	·•									Atter	
	epth		ember 1979	Pen	etrati	on Data			grave:	11	Grain sand	Size	1	silt		hlau	Limit (% wa	
		Log	Description	N	qc	_, Rf	, Cu	D50	Marave.	-1 . c	m	, f	, c	, m	, f	r ray	, Wp	
Γ.		F. 1																
1	1	+ +			22 22	3.6 3.6	-	.029	-	-	3	28	25	16	7	21		- 1
	2┥	[+]	fill, fine sand	1	12	2.2												İ
] 3	3-1	[+]			24	1.1											-	
4	,-	+ +		-	12 34	3. 3												1
	1		brownish grayish	6	34 46	1.2												1
1 6	2		g1 ay 1311	一	44 44	0.6 0.3	12	.070	-	-	1	70	13	7	3	7		- 1
	12		(01)	 	34	2.7	 -	.060			-	50	30	8	4	8	-	
8			(SM) silty sand and sand, (ML) silt near		24	0.6								J	•	Ū	1	1
ľ			bottom, dark reddish	12	30	0.4	4.4	.094	-	-	1	69	22	3	2	3	1	
	1		brown (5YR3/4) at top, dark yellowish brown	12	36 48	1.9 2.8	-	.099	-	-	-	76	8	5	6	4	ŀ	1
10	13		(10YR4/4) near bottom,		56	2.1											1	
	-		loose-medium dense,		72	2.0												
12			well-poorly sorted	_	74 50	2.5 3.2	2.4	005			2	70		_				
	4			13	44	3.0	1	.095	-		2	78	15	2	1	1		
14	1 .			\vdash	40 38	2.0 2.1	4.5	.089	-	-	1	70	19	4	2	4		
1 4					44	1.8	3.2	.072	-	-	-	63	29	.3		5		
	1			ĺ	26	3.1	7 -	044						_	_			
16	5			—	14	5.7	3.5	.066				55	35	5	3	2	-	
	1			7	14 24	1.0 4.4												
18	1				22	1.8	-	.025	-	-	-	4	50	23	7	16		
,	-				22	3.0	-	.001	_	_	_	1	3	11	23	62	25	73
20	6		(CH) clay, top reddish	_	16 22	4.2 3.0	 										-	
			brown (5YR4/3), bottom half dark brown	13	28	1.9												
122	1 1		(10YR3/3)		30	25.8												
22	1 1		(SP) very fine sand		44 108	3.0 1.2	3.0	.120	_	_	9	76		1	r			
	7		dense, well sorted, _coarsens_downward		112	3. 5	3.0	.120			9				3		1	\neg
24	1		coarsens downward	15	26	2.1												
	┨ ┃		silty clay, reddish		40 22	2.3	-	.002	-	-	-	16	10	12	14	48		
26	8		brown (5YR3.5/3), stiff carbon-rich zone at		32 32	3.0 1.7												
	1 1		7.6 m		32	1.3	-	.002	_	_	_	2	3	14	26	55		
]				34 20	2.0											1	1
28					180	3.3 2.0											1	1
	9		(SM) silty sand, poorly		116	2.0 2.8	17	.125		-	16	63	8	4	3	6	ļ	
30	1		sorted, dense,		116 120	2.8 2.2	-	.090	-	-	8	62	14	4	4	8		
	1		interbedded clayey silt 2-3 cm thick		150	2.0												
32			- o om onion		116	3.0												
	10				<u>48</u> 36	2.8												
					96													
34	1																	
	1																	
36	11		}				ļ ——										+	
	1																	
38																		
40	12																1	
70																		
	1																	
42	1 1																	
	13																	
44																		
]							59										
46	14						ļ										 	
'	7																1	
1.]																1	
48	1																	

			Atterberg
DATE December 1979 Grain Size			Limits
Depth Penetration Data gravel sand ft m Log Description N qc Rf Cu D50 c m f	silt c m f	clay	(% water) ,Wp Wl
1 18 8.9 20 5.3 40 5.7			
10 6.7			
2 3 10 6.7 10 6.7 10 6.7 10 6.7 16 2.2 18 0.7 16 2.5 16 2.5 16 2.0 20 3.3 4.4			
18 0.7			
16 2.5			
$\begin{bmatrix} 6 \\ 2 \end{bmatrix} = \begin{bmatrix} 16 & 5.0 \\ 20 & 3.3 \\ 46 & 1.4 \end{bmatrix}$			
1 1 66 2.0			
1 °1 1 56 2.4			
68 1.4			
10-3 58 3.4 42 2.5			
40 2.3			
12 46 2.3 38 2.8			
38 2.8			
10-3 12- 4 12- 4 14- 14- 14- 14- 14- 14- 14-	,		
1 1 1 1 20 601			
16 5 20 4.7 20 4.7 24 3.3			
28 1.9			
28 1.9 34 2.4 30 3.6 20 4.7			
30 3.6 20 4.7			
$\begin{bmatrix} 20 & 4.7 \\ 24 & 3.3 \\ 26 & 7.7 \end{bmatrix}$			
160 0.8			
160 2.0			
36 6.3			
10 6.7			
1 24 2.8			
26- ₈ 32 2.5 24 2.2			:
1 28 19.0			
120 0.9 140 2.3			
1 10 1 40 4.7 1			!
30-9 44 3.0			
68 2.7 26 5.1			
$\begin{bmatrix} 32 \\ 10 \end{bmatrix} \begin{bmatrix} 20 & 6.0 \\ 16 & 0.8 \end{bmatrix}$			
10 38			
34-			
36-11			
38-			
12			
42			
13			
60			
46-14			
48-			
15			





<u> </u>	LOCATION DATE Ja	.10N Ja	N El January	Centro- 7 1981		Table 12.		Log of Heber		Road 11	•		-	Grain	SIze				7	Atterberg Limits	
De	Depth It m	1.0.E		Deseript fon	it fon	_	Penel	Penetration Dat N , qc , R <u>f</u>	Data Rf	Cu) 1050	grave1	ບ	sand m	-	_	silt		rlay	(% water)	
	- 2 2	* * * * * *	fill dry,	l. loose.	very	fine,		30 38 38 32	2.7												
			very f		well ish br	sorted	_ ∞	22 22 20 14 60 14 60 14		1.8	.102			-	68	:					
-	<u>`!</u>		(5YR4. brown ripple horizo dense	4	to dark R4/4) 1-1.4 beds 1.	m 4-3.8	28	170 180 170 144			· ·										
63	10-3							140 160 184 184	wawaw wa400	4.0	.110	ı	ı	ъ	77		1			; ; ; ;	
	7 4	W W	silt,			gray	13	190				ı	1	,	19						
16	5.		upper	cont	, sharp act	i		1882	707												
18			clay, (brown, firm	ਰ	ark grayish (10YR4/2),		4	2005	0005 n												
20-	9.	W						240	000												
22	2-							132 132 38 38	2.8												
24.			PH 118 19810		-			22 24 12 24 14 44	4 M O M								-				
*	<u>×</u>							110 84	3.0 2.9								!	4			



LOCATION El Centro- Table 14. Log of Heber Road 13.

	ATE		Table	14.	Log of	Heber	Road	13.									Atterber
	epth		inuary 1901	Pene	etration	Data			gravel		Grain sand	Size	1	silt		hlav	Limits (% water
		Log	Description	ı N	, qc	Rf	, Cu	_ D50	Braver	С		, f	, c	, m	, f	r ray	Wp Wl
	T	+ +															
	1 2	[+]			34 8	6.7 13.3	1										
		[+]	fill, moderately-poorly		24	3.9	1										
	3 1	[+]	sorted, very fine sand	L	24	4.4											
- 1	4-	+ +	reddish brown (5YR4/4)	10	22 18	3.6											
]	. * .		10	22	3.0 3.6	10	. 111			-	7.0					}
1	.]	1.			18	1.5 3.3	10	.111	-	-	7	73					
(5 2				20												
	1			5	20	1.3											
8	3-			<u> </u>	16 16	3.3 1.7											1
]				18	3.0											
1,,	3		sand, very fine to		18	3.0 3.7 3.3											1
110	"		silty sand, reddish		20	3.3	7	.120	-	-	8	78					
	1		brown (5YR4/3) dilatant		14 18	2.9 4.4											
12	2┥				22	3.0											
	+4				24	3.3											
14	1			_	26	3.6											
14	']			5	28 3 6	3.8 2.6	10	. 140	_	_	12	72					1
	1				22	7.3	10	.140	-	-	12	12					
16	5				10	7.3 2.7											
	13		silty clay to clay		24	2.8											
18			dark reddish gray (5YR4/2)	9	12	6.7											
1	'	Ġ	(31K4/2)	_	14 24	4.8 13.9											
	16				30	2.7											
20	6			_	200	2.7											
	1		sand, very fine,	32	240 224	1.8 2.1											
22	2		uniform, no bedding		170	2.1											
	7				126												
	1																
24	1			11													
	1		silty clay to clay	11													
26	l ₈		reddish brown (5YR4/3)														
	[(31,473)														
		娼															j
28	1	逜		28													1
	1,9		sand, fine, well sorted														
30																	
	1		_														
,,			•														
32	10																
1	1																
34	1																
	┨																
36	11																
100																	1
38	1																
	12																
40	112																1
																	1
1																	1
42	1																
	13																
44	1										•						
]							6 5									
1.6	14																
140	714					I											
	1					1											
48	1																

LOCATION El Centro-Table 15. Log of Heber Road 14. Atterbei DATE January 1981 Grain Size Limits Penetration Data Depth gravel sand silt clay (% water Description qc R_f Cu D50 f Wp W1 ft m Log 18 14 26 22 14 18 14 12 10 10 42 36 16 12 10 3 16 20 26 12 18 18 22 14 30 40 30 18 16 22 14 18 18 22 32 230 210 20 220 22-24. 26-28 30 32 10 34 36+11 38-40. 42. 13 66

48-

LOCATION El Centro-Table 16. Log of Heber Road 15. Atterbe DATE January 1981 Grain Size Limits Depth Penetration Data gravel sand silt clay (% wate ft m Log D50 Description Cu Wp W] qc R_{f} m m 2.7 34 34 3.5 fill, fine sand, some 22 26 4.8 silt, gastropod shells 2.1 16 1.7 3.3 2.5 3.8 12 1 19 16 28 2.1 64 82 3.9 66 3.2 96 76 2.1 10 3 sand, fine, well sorted 40 3.0 reddish brown (5YR4.5/3) 34 3.1 34 64 3.9 2.5 2.7 2.5 .112 2 89 12-64 3.1 2.7 56 54 50 42 2.9 3.8 30 4.9 16 24 3.9 clayey silt at top 26 4.1 18 dark brown (7.5YR4/2) 2.9 28 _ _ to 2.9 3.1 2.1 28 26 26 22 20 silty clay at bottom reddish brown (5YR4/3) 4.2 22 3.0 sand, fine, uniform 1.6 170 22 12 dark brown (7.5YR4/4) 88 silty clay with 5 mm beds of silt, dark grayish brown (10YR4/2) 2618 28sand, fine, well sorted 24 reddish brown (5YR4.5/3) 30-32. 10 34 36+11 38-12 40 42. 13 44. 67

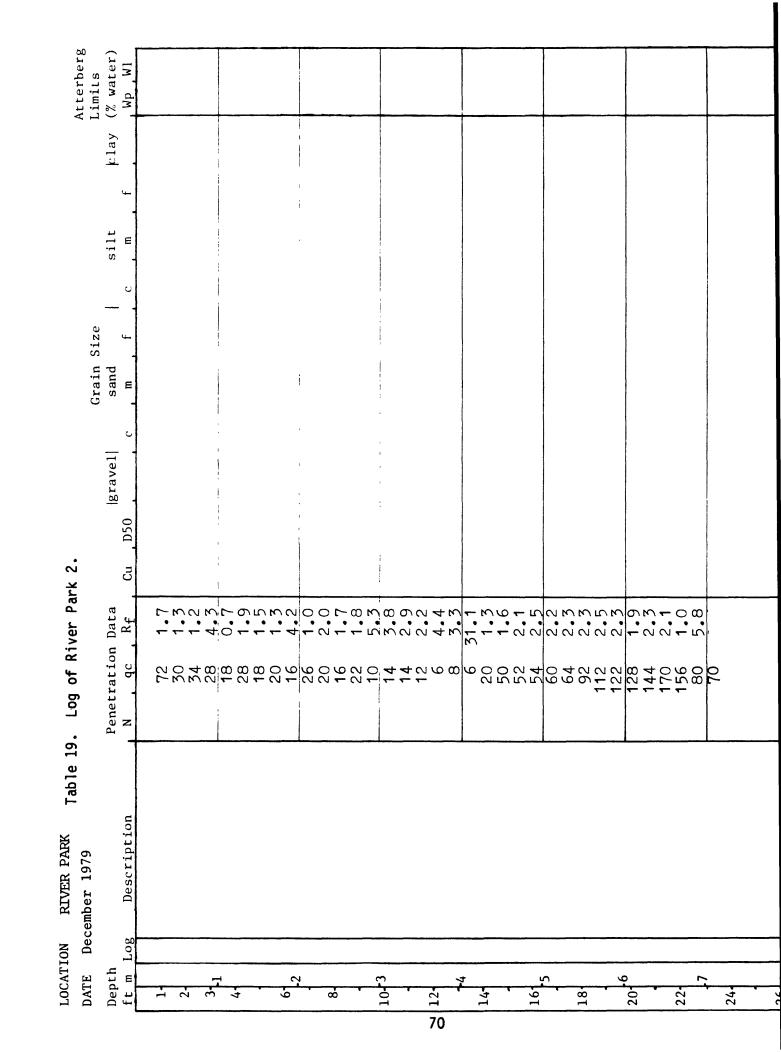
46+14

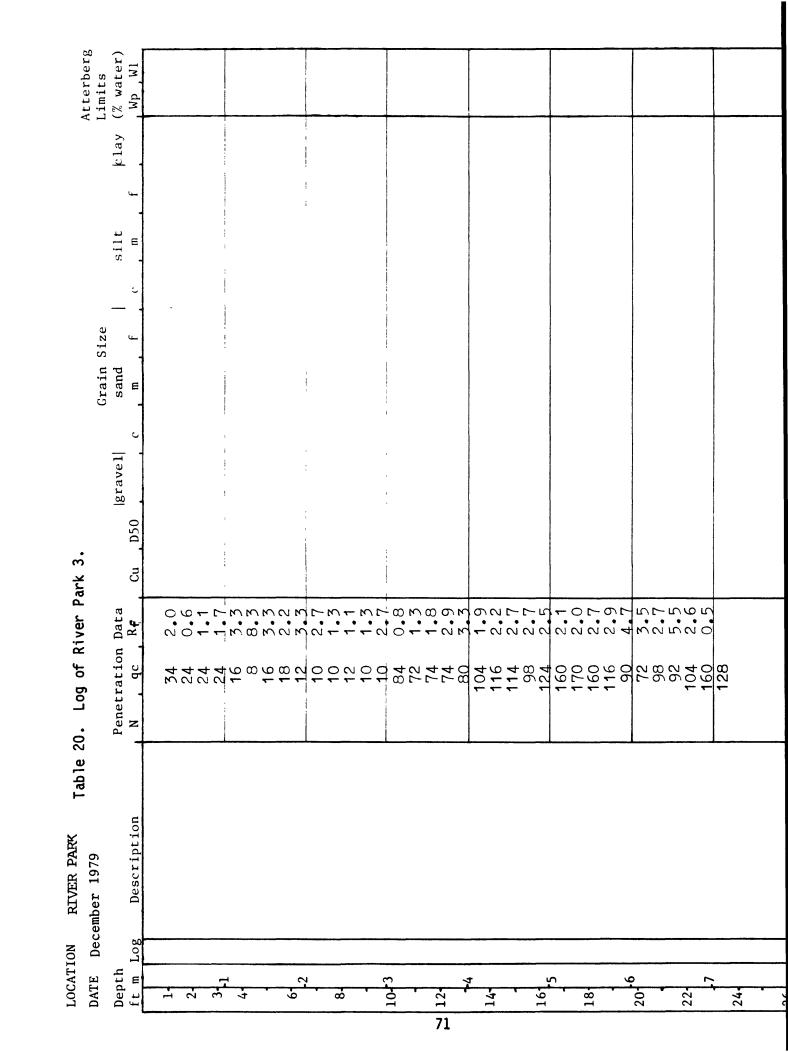
48-

LOCATION El Centro- Table 17. Log of Heber Road 16.

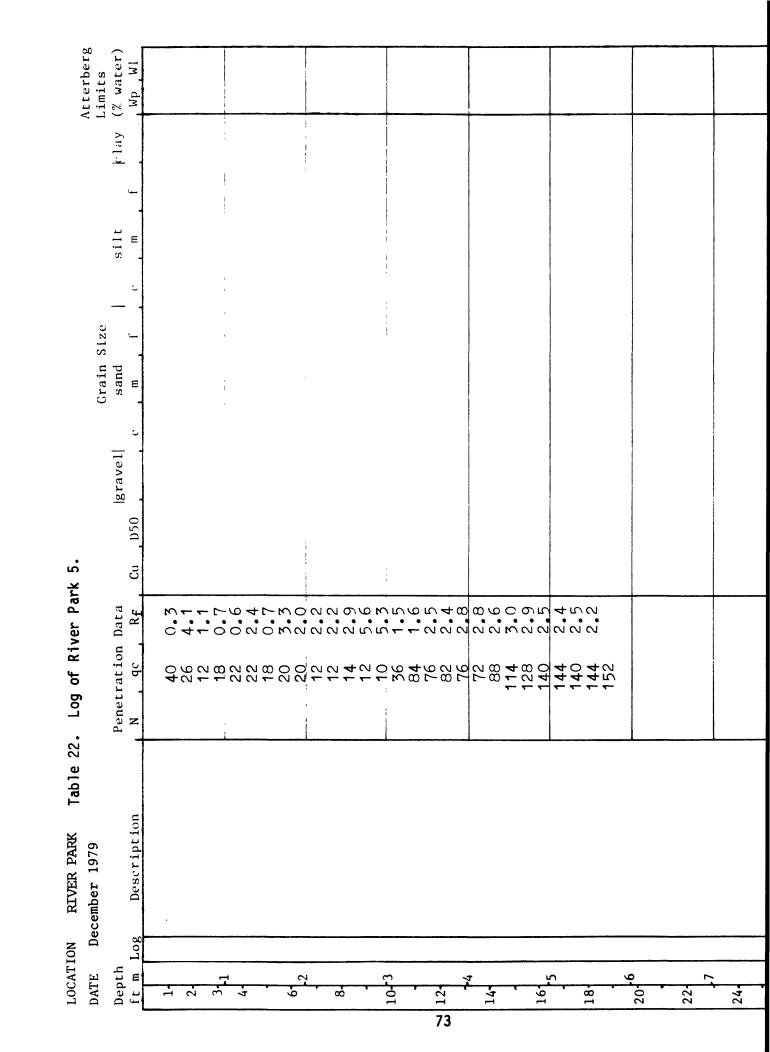
1	ţlay f	Atterber Limits (% water Wp Wl
ft m Log Description N qc Rf Cu D50 c m f c m 1	f lay	Wp Wl
1		
3 1 10 10 10 18 18 28 36 58 60		
3 1 10 10 10 18 18 28 36 58 60		
18 28 36 58		
18 28 36 58		
6 36 36 58 60 72		1
58 60 72		1
1 o 1000001		
8- 78 66		
8- 72 78 66 66 68		
10-3		
1 54		
12 50		
1, 14		
14-		
10-3 12-4 14-14-16-16-16-16-16-16-16-16-16-16-16-16-16-		
[5] 888		
18 22 26 22 26 22		
		
20 24		
24 28 170		
104		
24 18 32 22 22 22		
32		
26-8		
26 8 22 18 30		
28.		
220		1
28 220 170 180 116 70 48 30 30		
116 70		
32 48 30		
10 30		
34		
36-11		
38-		
40 12		
42		
44-		
68		
46-14		1
48-		

Atterberg Limits	(% water) WP W1			26 65		27 80						
Ą	play (7	36	6	99	2					
	Ŧ		m (21	7	17	H				_	
	silt m		TI V	20	25	œ	H	9	14	7	9	
	C		39	21	47	4	7					
Size	<u> </u>		39	73	12	м	53	4	52	83	9/	
Grain S	sand		Н		1	7	36	02	34	10	18	
9	ر ن		ı	1 1	1	1	ı	-	- I	ı	ı	
	grave1		ı	1 1		1	ı		i i	ı	ı	
	g D50	,	.051	.004 004	.026	•001	.165	Väl	.170	.115	.145	
. i	Cn		1		1	ı	3.3	7	. 4 . 8	1.9	2.0	
r Park 1	Data Rf	0220	MC 0	0.00	0 M		M000	2	00000	∞∞-0-	2022	
Log of River	Penetration N qc		22 18 22 14	1			20 20 9	- 1	128 108 90 86 70			
Log	Penetr N		. ~ .	1	5	1 =	1	İ	1 1	21	21	
ION RIVER PARK Table 18. December 1979	og Description	surface (ML) sandy silt, very	dark grayish brown (2.5%3/2), moderately sorted, very loose, moist, slightly plastic abundant roots		silt interbedded, dark grayish brown (2.5%3/2)	<pre>H) clay, dark gray '1), plant material abundant, very</pre>		(QD) fine gand moll	sorted, very dark gray- ish brown (10YR3/2), 15-18 ft, brown/dark brown (10YR4/3) 18-25',	shells present, dilatant near 17-21'	*	
LOCATION DATE Dec	oth m L	* 5 + 1 + 1 + 1 + 1 + 1 + 1 + 1 + 1 + 1 +		·		/		 •			, , , , , , , , , , , , , , , , , , ,	
)7 [[(De _F					69 12 1.	14-	16.	18	20-	24.	

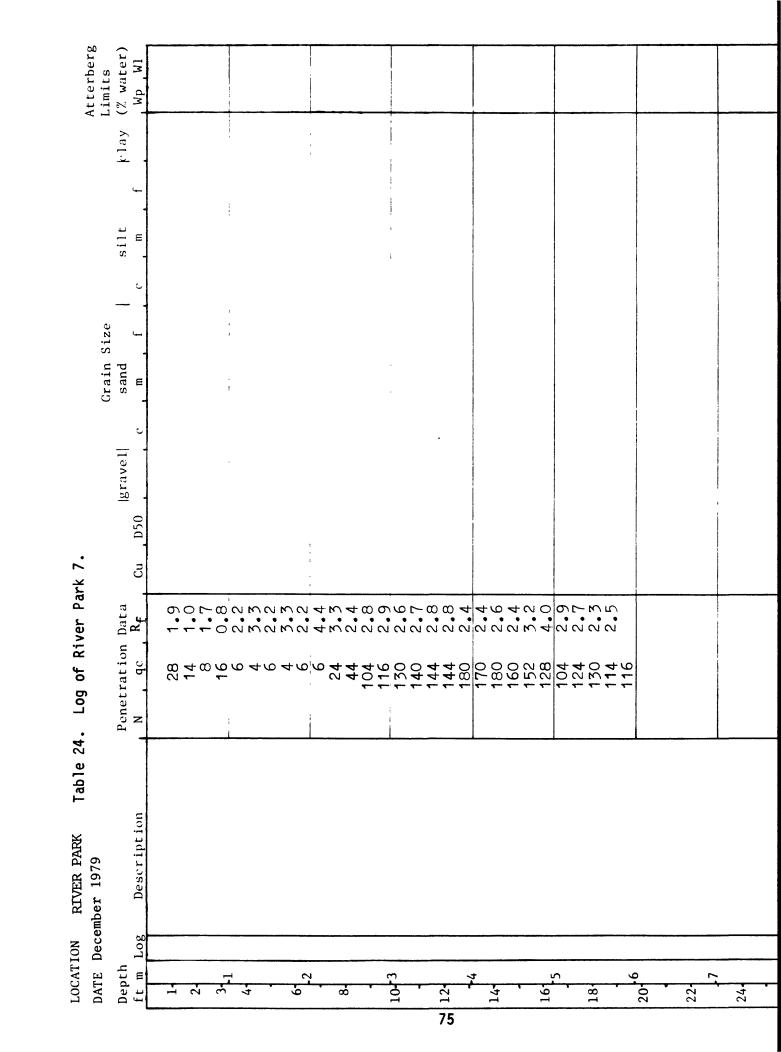




LOC.	LOCATION	ON RIVER PARK Table	le 21.	Log of	River	Park 4	•									•	
DATE		December 1979							J	Crain S	Size				,	Atterberg Limits	erg
Depth	th		P	Penetration				gravel				•.	silt		rlay	(% water)	er)
ft.		Log 🔻 Description	_	N qc	F.	Cu	, 05u	- <u>-</u>	- ن	Ε	J	٠	ε			Wp	Wl
-			/us	,		,	į										
, ,	<u>padidi</u>	loose, dilatant, very		040	•	0 80	•071	i	ı	-	28	27	2	m	2		 .
2-		cohesive		39	• •												
Å.	<u>::::::</u> →	[ML] clayey silt, very		8.5	0,0			ı	1					:		:	T
7	1,1,1	silty at bottom, some	<i>a</i>)	4γ α	•	V											-
	بلتك	sand, maybe thinnly		5	• 1		.010	ı	ı	ı	6	25	29	20	17		
	<u>1.11.1</u>	interpeaded, Silt is		22													
٥		[MH clay, brown/dark		18	0.7								•	•	(
	ינה) פֿנ. נ	\Rightarrow brown (7.5YR4/2), yellow	Llow	16	4.2	1	100	ı	ı	1	1	ı	4	22	7.5	27	74
œ		concretions, very firm	- -	10	2.7												
	Lide".	(MH) clay, very dark		12	2,5		נסס	ı	ı	1	1	~	'n	00	99	27	
Y	<u> </u>	grayish brown (loYR3/2)	,2)	ω <	7.	l 	100.	Ì	l	l	l	า	n	07	00		
-2	3	Filorganics common, soft		9 1	12.5												Ţ
2				<u>۵</u> ۵	4.	3.5	.110	ı	1	20	89	4	4	٦	c		
	•			98	- 2	o C	120	ı	1	1	ď	1	~	-	C		
12-	•			82	2.6	o• 7	. 140	I	l	,	0	•	n	4	۷		
}	::::::::::::::::::::::::::::::::::::::	(SP) fine sand, well		84	2.5	3.1	.135	1	1	11	11	9	2	1	2		
	<u></u> r	sorted to moderately		88 6	0,0 W,0												····
7	<u> </u>	sorted, brownish above	e -	104	0 10	2.7	.140	ı	i	11	78	7	2	Н			
	<u></u>	13 , grayish below 13		112	3,2	2	145	ı	ı	17	73	v	0	_	_		
16		gastropod shells at 1	13	120	2.7) 1	C# T •			À	2	o	١	4	4		
· •	<u> </u>	rilling bed	mes	132	2,8	2.7	.145	1		14	9/		10				Ī
7	نننن	harder at 16', better		124	N C	3.6	.145	ı	ı	17	70	2	4	Н	т		
2	:::::	recoverey at 16-20		120	, K	6.5	.140	1	ı	15	29	ω	4	7	4		
	<u>::::::</u> y			122	3.4	0.9	.135	1	ı	13	63	15	m	2	4		
20-	.	*	T	138	2.4												
				150	2.0	- · · · <u>-</u>											
2,2	-			160	N C												
				156	0 0												-
	`		<u> </u>	160	2.5												
24-				160 156	200												
				156	2.2												
26-8				180	2.2	-					!	-					



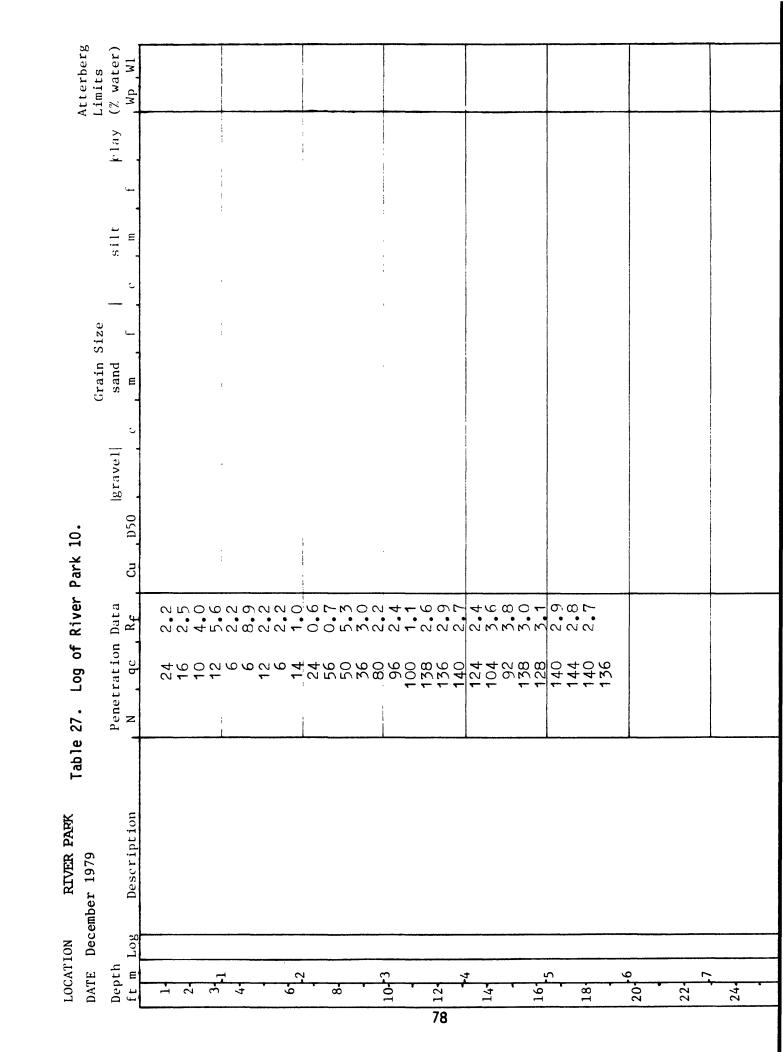
DATE		December 19/9							Grain	Size					Limits
Depth	'n		Penetration	Data			gravel				_	silt		rlay	ದ
ft m	Log	og Description	N qc	Rf	Cu	D50		J	Е	ų,	٠	ш	f		Wp Wl
1	<u> </u>	(ML) sandy silt, dark brown (7.5YR3.5/4), very	66	2.4	ı	.044	2	1	4	21	40	6	3	17	
-5-	+ +			01											
,	+	(SM) gravelly silty sand	~ ~												
7	+		;	2.7		1	1	1	, 1		, (1	!	:	
,	<u>+</u> -	and reddish brown (5YR	10	2.7	ı	.210	24	12	14	14	14	10	9	9	
	<u>.</u>			w 10 10 10											
નુ	+	artificial fill	4 4	0.0	ı	.035	œ	9	œ	12	29	15	7	15	
<u> </u>			10	1.3	1	.033	ı	7	4	15	41	16	9	16	
&	<u>i.i.</u>	clayey silt, mixed,	ه و	25)		I	,	ļ	,	;			
	Ш	probably interbedded	10 8												
, ,	L			•					ı				1	1	
		(CL) clayey silt and	28		ı	.044	i	ı	ı	27	46	11	5	11	
•		sandy silt, black	9	5.5 - 2	ı	.010	i	ı	ı	5	26	26	15	28	·
17.	iji		9	2,2											
7		(SM) silty sand, brown/		2-1	1	060.	ı	ı	2	64	22	4	4	4	
14			56	2.4					, ,		-	Ų	ų	r	
		,	92	0, 0 10, 1	ı	c60.	i	ı	r;	9	07	٥	n	•	
164		£.	99	2.5		:									
_			09	2.7											
18			130	и Г											
,			144	, -											
9			134												
•															
22-															
7															
24-															
·					_										



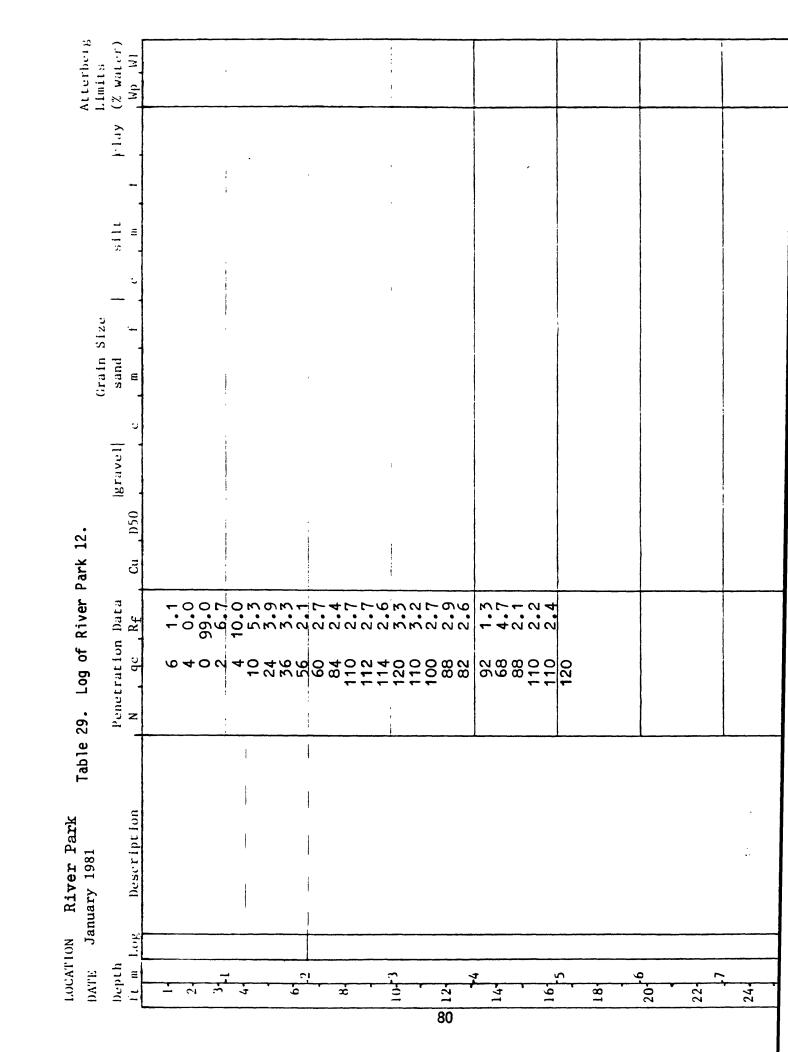
Atterberg (% water) 82 77 7 Limits Μp 24 17 rlay 10 70 S \sim į 20 ω 7 2 S 2 silt α ~ 2 Ε 4 \sim 47 9 ~ α S σ 73 75 25 73 69 80 Grain Size sand 12 14 15 15 19 8 i ŧ ı ŧ gravel ŧ ı ŧ ŧ .045 .140 .140 .150 .150 .130 .001 D50 Table 25. Log of River Park 8. 7.5 1.9 7.0 3.1 1.9 Cu W004 -000 2.0 Penetration Data ďc 94 446 041 444 150 041 041 041 041 144 152 128 124 4400040 144 z (SP) fine sand, brownish sandy silt, brown, well sorted, very poor recoverey between 5 to grayer bewteen 8 to 15 dark brown (7.5YR4/4), (MH) clay, brown, dark brown (7.5YR4/4), firm represent interbedded clayey silt and sand poorly sorted, may between 5 to 8 ft, Description RIVER PARK DATE December 1979 ਉ LOCATION 120 Depth ft m 10-3 14 161 6 12 22-ထ် 18 24 76

LOCA' DATE	r10	N RIVER PARK Table 26. December 1979	Log of	River	Park 9.	•			7. 2. 3.	0.130					Atterberg Timirs
Depth ft m	h Log	g Description	Penetration N qc ,	n Data Rf	Cu	ا 050	gravel	, ,	sand	, 12e	;	silt m	f	rlay	(% water)
1-	+ + + +		,	7×1 2×2	1	.034	ı	ŧ	1	22	42	11	∞	16	
£ 4 -	· + + + • • • • •	and sand, uniform, not interbedded, brown/dark brown (7.5YR4/4), very poorly sorted ,loose -	0 4 4 4	0 m m m	ı	• 030	1	ı	т	. 56	32	14	∞	19	
6-	<u> </u>	(SM) gravel, sand, silt	10	5 0 8	1	.085	20	11	10	15	17	11	9	10	ļ
. %	+ + +	and clay, extremely poorly sorted, BLACKTOP artificial fill, black	045	0.0	ı	.140	15	7	18	18		38	æ		
	+ + +	+ +	26 48	2	ı	.325	29	12	17	14		28	œ		
10-5	<u>.</u>	(SP, SM) sand and silty	27 77 77 77 77 77 77 77 77 77 77 77 77 7	00000											
16-4	_	well to gd, 10 to conserved commish and 14 to 20	88 102 102 116	72002	2.9	.160		г	29	09	m	7	-	4	
18		sand and silty sand dense	126 130 144	22.2	5.7	.180	10	10	30	42	9	4	7	7	
			148	2.3	ı	080	11	9	13	27	23	œ	m	6	
20-7	<u>1</u>	+	156 170 170	2.0											
24-															

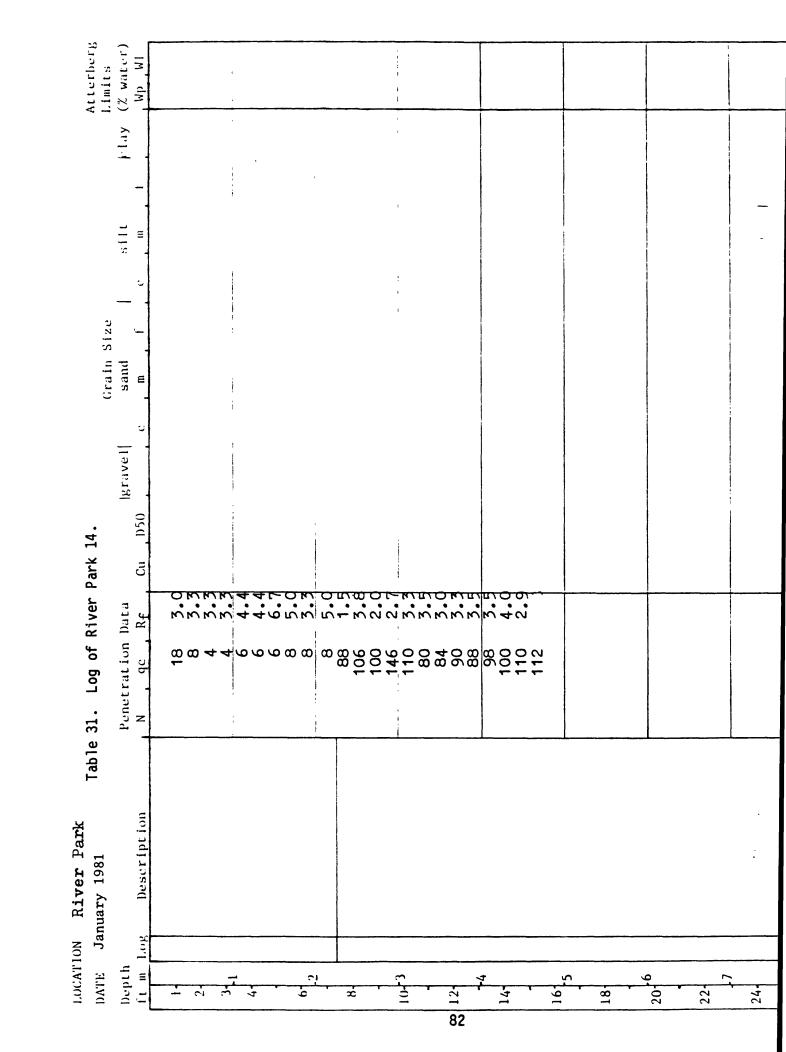
ı



DATE		December 1979						J	Crain S	Size				W _	Atrerberg Limits	50
Depth ft m		og Description	Penetration D	Data Rf	Cn	^{ह्य} . 030	gravel	- د			თ . ა	silt m	<u>.</u>	klay ((% water)	(r. 1
1-2-2-		(CH) clay, reddish brown firm, roots very abundant		r w w w		• 002	ı	ı	ı	н	4	19	27	49	32 56	9
4				100-r	ı	.001	1	!	. I		, 7	ω	50	69	31 67	7
9	7	(ML) silt, moderately to poorly sorted, maybe	10 17 4 47	/W:4C	; ; ;	.042	1	ı	1	14	65	œ	4	6		
œ (2010	4.1	.128	ı	1	11	92	2	٣	7	m		
10	n T			w 0	2.5	.145	ı	1	16	74	7	7	4	,		
12-		(SP) fine sand, well	156 144 140 140	10 L 10 L	1.8	.170	1	i	26	70	4					
14-1	7	8 and 13', grayish between 13 and 20'		100	1.8	.155		1	20	9/	4					T
16.		thel]	1 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	000	1.8	.158	ı	1	56	70	ю	н				
	ر.			7-	1.9	.165	1	1	26	69	2					Τ
ă				90	2.1	.188	1	ı	44	52	4					
0 1			144	رح —	2.2	.195	ı	7	46	49	e					
	<u></u> 9		-		2.4	.210	1	ı	55	41	4					
20-	3	*														
.77	- 2					197. Annual 197.					· January Mark					T
24-																



LOCATION River Park Table 30. Log of River Park 13. Atterberg DATE January 1981 Grain Size Limits Depth Penetration Data gravel sand silt clay (% water) Cu D50 ft m Log Description , qc Rf m Wp W1 40 3.0 30 3.1 2-10 4.0 311 silty clay, dark brown 10 (7.5YR4/2)8.9 6 8 5.0 6 5 53.3 5.0 1.5 2.7 2.7 2.6 4 6 -2 62 74 8. 54 17 84 88 10 | 3 3.3 0.2 100 110 17 156 2.5 3.0 3.0 12-132 124 2.4 134 sand, fine to coarse, 132 well sorted, dark 3.2 3.1 114 brown to grayish brown 18 106 (10YR4/3) to (10YR5/2) 120 2.7 16 coarsens downward, gastropod shells are 2.6 116 common 100 2.4 18 88 108 2.7 14 102 20 144 1.4 130 170 2.6 1.7 2.0 22-180 156 27 88 180 1.9 24. 180 2.1 160 1.5 108 2.0 26 164 1.4 94 2.3 26 168 2.4 28-160 3.8 180 196 2.7 30 82 4.4 36 36 6.3 3.7 32 silty clay, dark brown 36 (7.5YR4/2)3.6 3.3 30 28 34 30 6.7 170 2.4 200 3.8 36+11 96 230 2.5 224 38 40. 42 13 81 46+14 48



LOCATION River Park Table 32. Log of River Park 15.

	OCAI ATE	N River Park January 1981	Table :	32.	Log of R	River	Park	15.									Atterberi
	epth	undary 1501		Pene	etration	Data			grave1		Grain sand	Size	ı	silt		b1av	Limits (% water)
	t m	og Description		N	, qc		Cu	D50		с	, m	f	· c	m	f		Wp W1
:	1	sand, fine, and s	andy	_	42	1.0											
ı	2	silt dark brown (7.5YR4/4)		7	32	1.9	-		-	-	-	21					
	3 1				20 20	2.7											
	,√*	를 를 clayey silt and a	lter-		14 12	2.9	ol .										
	-	nating thin beds	(1 cm)	1	14	2.9	-		-	_	-	2					
1	5	of clayey silt an sand, dark grayis		_	10 12	1.3 0.6	-		-	-	-	13					
	2	brown (10YR4/2)			18	1.5											
8	3-	clay, alternating		3	6 6	4.4 4.4											1
	1	of brown, gray an green, soft, orga	nics		6 6	4.4			-	-	-	1					
10) 3				40	2.0											-
	1			7	8 8	2.3	1.1	.170	-	-	31	66					
12	24				86 80	2.6	1.6	. 149	-	-	12	85					
	+4				78	2.6	l	147				70					
14	,-			21	80 100	2.8	2.0	. 147	-	-	16	78					
	1				104 110	2.7											
16	, l	sand, fine, well		21	112	3.0	-	.138	_	-	14	67					
	15	moderately sorted dark brown (10YR4			130	2.8											
18	3	below 5 m dark gr			142 130	2.8 2.6											
	1	brown (10YR4/2)			150 140	2.5 2.4											
20	6			35	120	2.8											
	1				146 152	2.6 2.4											
22	:-				190	2.5											
	7				200 154	2.7											
24				31	164	2.4	2.2	.210	-	-	54	44					
	-				180 160	2.6 2.3											
26	8				180	2.2											
	-				156 176	2.3 2.4											
28	H				180 168	2.4											
	-				160	2.4 2.8 2.6											
30	-				158 140	1.7											
	1				144	3.6 2.2 3.3 2.7											
32	1	A A			100 160	3. 3											
	10				196										····		
34	1	8			152 126	2.0 3.2 3.0 3.4 3.4 3.0											
	┨				164	3.0											
36	11				196 164	3.4											
	1				200	3.0											
38	1		İ		270	Ì											
	12							·					- 				
40																	
	1																
42			İ														
	13		ļ								·····						
44	1																
	1						1	33									
46	14		}				-										
	1																
48	1																<u> </u>